Community Services Department

Planning and Building

ADMINISTRATIVE PERMIT APPLICATION

Community Services Department
Planning and Building
1001 E. Ninth St., Bldg. A
Reno, NV 89512-2845

Telephone: 775.328.6100

Washoe County Development Application

Your entire application is a public record. If you have a concern about releasing personal information, please contact Planning and Building staff at 775.328.6100.

Project Information	**Staff Assigned Case No.:**
Project Name: Lipscomb Garage	
Project Description: Detached Garage	
Project Address: 195 E Sky Ranch Blvd	
Project Area (acres or square feet): 4000	
Project Location (with point of reference to major cross streets **AND** area locator): E Sky Ranch Blvd and Omni Dr	

Assessor's Parcel No.(s):	Parcel Acreage:	Assessor's Parcel No.(s):	Parcel Acreage:
534-113-02	1.002		

Indicate any previous Washoe County approvals associated with this application:
Case No.(s). N/A

Applicant Information (attach additional sheets if necessary)

Property Owner:		**Professional Consultant:**	
Name: Aaron Lipscomb		Name: Element Engineering	
Address: 195 E Sky Ranch Blvd		Address: 3960 Glenview Terr	
Sparks, NV	Zip: 89441	Reno, NV	Zip: 89503
Phone: 775-686-0310	Fax:	Phone: 775-762-5461	Fax:
Email: bdnatalied@gmail.com		Email: andrew@elementengineer.com	
Cell: 775-686-0310	Other: mobile	Cell:	Other:
Contact Person: Natalie Davidson		Contact Person: Natalie Davidson	
Applicant/Developer:		**Other Persons to be Contacted:**	
Name:		Name: Natalie Davidson	
Address:		Address: !95 E Sky Ranch Blvd	
	Zip:	Sparks, NV	Zip: 89441
Phone:	Fax:	Phone: 775-686-0310	Fax:
Email:		Email: bdnatalied@gmail.com	
Cell:	Other:	Cell:	Other:
Contact Person:		Contact Person: Natalie Davidson	

For Office Use Only

Date Received:	Initial:	Planning Area:
County Commission District:		Master Plan Designation(s):
CAB(s):		Regulatory Zoning(s):

Administrative Permit Application
Supplemental Information

(All required information may be separately attached)

1. What is the type of project or use being requested?

Detached Garage

2. What section of the Washoe County code requires the Administrative permit required?

110.306

3. What currently developed portions of the property or existing structures are going to be used with this permit?

N/A

4. What improvements (e.g. new structures, roadway improvements, utilities, sanitation, water supply, drainage, parking, signs, etc.) will have to be constructed or installed and what is the projected time frame for the completion of each?

N/A

5. Is there a phasing schedule for the construction and completion of the project?

Construction to begin as soon as permits are approved

6. What physical characteristics of your location and/or premises are especially suited to deal with the impacts and the intensity of your proposed use?

the garage will decrease weeds on the property and remove vehicles and trailers f

7. What are the anticipated beneficial aspects or effect your project will have on adjacent properties and the community?

The Property will not have vehicles visible, add wind break to neighboring property

8. What will you do to minimize the anticipated negative impacts or effect your project will have on adjacent properties?

Keep construction to daytime hours to not disturb neighbors

9. Please describe any operational parameters and/or voluntary conditions of approval to be imposed on the administrative permit to address community impacts.

N?A

10. How many improved parking spaces, both on-site and off-site, are available or will be provided? (Please indicate on site plan.)

There will be indoor parking on this facility

11. What types of landscaping (e.g. shrubs, trees, fencing, painting scheme, etc.) are proposed? (Please indicate location on site plan.)

natural/ groomed landscaping

12. What type of signs and lighting will be provided? On a separate sheet, show a depiction (height, width, construction materials, colors, illumination methods, lighting intensity, base landscaping, etc.) of each sign and the typical lighting standards. (Please indicate location of signs and lights on site plan.)

N?A

13. Are there any restrictive covenants, recorded conditions, or deed restrictions (CC&Rs) that apply to the area subject to the administrative permit request? (If so, please attach a copy.)

☐ Yes	☑ No

14. Utilities:

a. Sewer Service	Septic Tank
b. Water Service	Great Basin Water Company

For most uses, the Washoe County Code, Chapter 110, Article 422, Water and Sewer Resource Requirements, requires the dedication of water rights to Washoe County. Please indicate the type and quantity of water rights you have available should dedication be required:

c. Permit #	N/A	acre-feet per year	N/A
d. Certificate #	N/A	acre-feet per year	N/A
e. Surface Claim #	N/A	acre-feet per year	N/A
f. Other, #	N/A	acre-feet per year	N/A

Title of those rights (as filed with the State Engineer in the Division of Water Resources of the Department of Conservation and Natural Resources):

unknown

Administrative Permit Application
Supplemental Information
for Care of the Infirm

(All required information, to include the physician's signed affidavit, is considered a public record and will be treated as such by Washoe County. Information may be attached separately)

1. Name of the Infirm:

Detached Garage

2. Name of Nevada licensed physician identifying the need for on-premise care and the physician's estimate as to the length of on-premise care required (attach physician's signed affidavit, form on page 11):

110.306

3. Name(s) of the Caregiver(s):

N/A

4. Describe the type and size of recreational vehicle or self-contained travel trailer that is proposed for use as a temporary residence of the caregiver. (Attach a site map showing the proposed location.)

N/A

5. Describe the arrangements/methods proposed for the temporary provision of:

   a. Water Service:

Great Basin Water Service

b. Sewage (Sanitary Sewer) Service:

Septic Tank

c. Garbage (Solid Waste) Service:

Waste Management

d. Electricity:

NV Energy

e. Natural Gas:

NV Energy

6. What will you do to minimize the anticipated negative impacts or effect your waiver will have on adjacent properties?

Keep construction to daytime hours to not disturb neighbors

7. What types of landscaping (e.g. shrubs, trees, fencing, painting scheme, etc.) are proposed? (Please indicate location on site plan.)

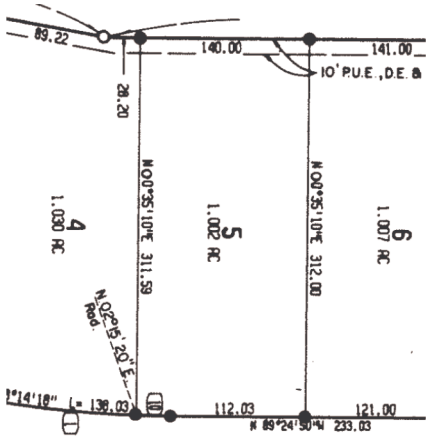
natural/ groomed landscaping

8. Are there any restrictive covenants, recorded conditions, or deed restrictions (CC&Rs) that apply to the area subject to the administrative permit request? (If so, please attach a copy.)

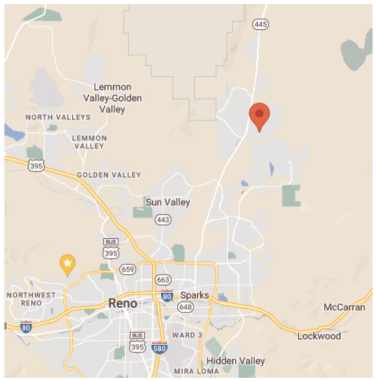
☐ Yes	☑ No

9. Community Services (provided and nearest facility):

a. Fire Station	Truckee Meadows Fire Department
b. Health Care Facility	Northern Nevada Medical Center
c. Elementary School	Spanish Springs Elementary
d. Middle School	Shaw Middle
e. High School	Spanish Springs
f. Parks	Eagle Canyon
g. Library	Spanish Springs Library
h. Citifare Bus Stop	N/A



1 RECORDED MAP



2 VICINITY MAP

REGULATORY ZONE DEVELOPMENT STANDARDS

Regulatory Zones	Title	Yards – Setbacks Front (feet)	Side (feet)	Rear (feet)	Maximum Height (feet)	Maximum Density (du/ac)	Minimum Lot Size	Minimum Lot Width (feet)
LDR	Low Density Rural	30	50	30	35	0.1	8 ac	250
MDR	Medium Density Rural	30	15	30	35	0.2	4 ac	200
HDR	High Density Rural	30	15	30	35	0.4	2 ac	150
LDS	Low Density Suburban	30	12	30	35	1	36,000 sf	120

SITE SURVEY NOTES

1. ALL WORK PERFORMED ON THIS PROJECT SHALL BE IN CONFORMANCE WITH THESE PLANS AND THE STANDARDS SET BY THE LOCAL BUILDING OFFICIAL.
2. NO DEVIATION FROM THESE DRAWINGS SHALL BE MADE WITHOUT PRIOR DOCUMENTED CONSENT OF THE LAND SURVEYOR. REPORT ANY PLAN DISCREPANCIES TO THE LAND SURVEYOR AND OWNER.
3. ALL DIMENSIONS SHALL BE VERIFIED ON PLANS AND DIMENSIONS SHALL BE VERIFIED WITHIN THE PROJECT PRIOR TO COMMENCEMENT OF WORK. DO NOT SCALE DRAWINGS, WRITTEN DIMENSIONS PREVAIL.

SITE SURVEY LEGEND

KEY	DESCRIPTION
◎	FOUND CAP
⬘	PROPERTY CORNER

* (E) = EXISTING, (P) = PROPOSED, CONC = CONCRETE, OH = OVERHANG (F) = FUTURE, CONT. = CONTINUATION, SB = SETBACK, HP = HIGH POINT (UG) = UNDERGROUND

SITE SURVEY
SCALE: 1" = 20'

DATE:
REVISIONS:

PE / PLS:
ANDREW HAMMOND
EXP. 6/30/25
8/24/23

ELEMENT engineering
3359 Peace Court
Sacramento 95826
775.762.5461
andrew@elementengineer.com

A SITE SURVEY FOR:
195 E SKY RANCH BLVD
SPARKS, NV 89441
APN: 534-113-02

SCALE: 1"=20'
DRAWN: AMH
CHECKED: AMH
DATE: 8/24/23
NOTES:

SHEET
SS

CALL 811 BEFORE YOU DIG
811
SAFE DIGGING PARTNER

PAD FOOTING CALCULATION

Footing / Foundation Design 1-15
2018 IBC references ACI 318-14

ELEMENT engineering

Project Information:

Risk Category:	II	
Site Class:	D	
Maximum Dead Reaction:	2800	lbs
Maximum Live Reaction:	9900	lbs

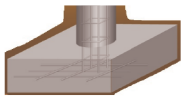
Resp. Mod. Factor (R):	-	
Importance Factor (I):	1.00	
ASD Factor:	0.7	

Effective Seismic Weights (W):

Dead:	2800	lbs
Live:	9900	lbs
Total:	12700	lbs

P_v = 12700 ← (lbs)

(ft) ↓ 3.00



S_B = 2000 ← (psf)

$FND_{bp} = (P_v + W_c) / S_B$

Concrete Foundaton Weight, W_c:

Width (sq), B:	3.0	ft
Conc. Unit W:	150	pcf
Depth, A:	3.00	ft
Total (W):	16750	lbs

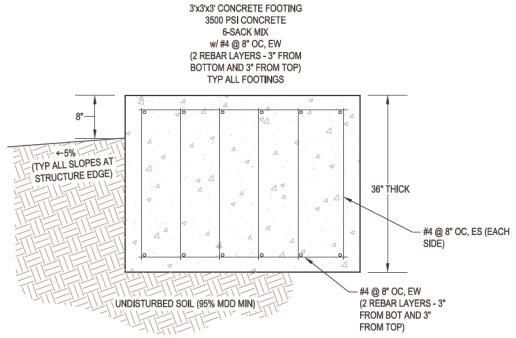
Required Foundation Size (square):

$Width_{required}$	<	$Width_{provided}$
2.9	<	3.0

True, Footing Size OK

Req. Rebar Specs:

Top:	#4 @ 8" OC, EW	Bar, (2" Clr)
Bot:	#4 @ 8" OC, EW	Bar, (2" Clr)
Vert:	#4 @ 8" OC, ES	Bar, (2" Clr)



PAD FOOTING SECTION

SCALE: 1" = 1'

FOUNDATION PLAN NOTES

1. ALL WORK PERFORMED ON THIS PROJECT SHALL BE IN CONFORMANCE WITH THESE PLANS AND THE STANDARDS SET BY THE LOCAL BUILDING OFFICIAL.
2. NO DEVIATION FROM THESE DRAWINGS SHALL BE MADE WITHOUT PRIOR DOCUMENTED CONSENT OF THE BUILDER. REPORT ANY PLAN DISCREPANCIES TO ENGINEER AND OWNER.
3. SLOPE ADJACENT SOIL AWAY FROM STRUCTURE AT 5% MINIMUM FOR 10'. RAIN DOWNSPOUTS SHALL BE INSTALLED TO ROUTE STORM RUNOFF INTO CONSTRUCTED SWALES AND INTO PERCOLATION BASINS.

PLAN INFORMATION

KEY	DESCRIPTION
•	NOT USED
○	NOT USED

* (E) = EXISTING , (P) = PROPOSED, HP = HIGH POINT, TP = TURN POINT
LP = LOW POINT, EW = EACH WAY, OC = ON CENTER

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
2*	E	0.6	1.6	0.7	1.5	2.9	6.0	6.7	14.0	−5.7	−8.4	0.5	−5.4
2*	A	−0.6	1.6	−0.7	1.5	−2.9	6.0	−6.7	14.0	−0.5	−5.4	5.7	−8.4

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
2*	E	−5.7	−5.1	0.4	−2.0	−0.6	−9.8	−1.3	−8.8	−1.8	−1.1	1.8	1.1
2*	A	−0.4	−2.0	5.7	−5.1	1.3	−8.8	0.6	−9.8	−1.8	1.1	1.8	−1.1

Frame Line	Column Line	Seismic_Long Horiz	Seismic_Long Vert	MIN_SNOW Horiz	MIN_SNOW Vert	F1UNB_SL_L Horiz	F1UNB_SL_L Vert	F1UNB_SL_R Horiz	F1UNB_SL_R Vert
2*	E	0.0	−5.6	6.7	14.0	5.6	13.6	5.6	8.0
2*	A	0.0	−5.6	−6.7	14.0	−5.6	8.0	−5.6	13.6

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind Press Horz
1	E	0.2	0.2	1.1	1.6	−1.8	−3.4	0.0	1.7	−1.6	−2.5	0.0	1.9	0.0
1	D	0.7	0.6	4.0	5.6	0.0	−1.3	1.8	−5.0	0.0	−0.7	1.6	−3.7	−2.5
1	B	0.6	0.6	4.0	5.6	0.0	−2.1	0.0	−3.7	0.0	−1.2	0.0	−2.8	−2.5
1	A	0.2	0.2	1.1	1.6	0.0	−1.2	0.0	−0.9	0.0	−0.6	0.0	−0.4	0.0

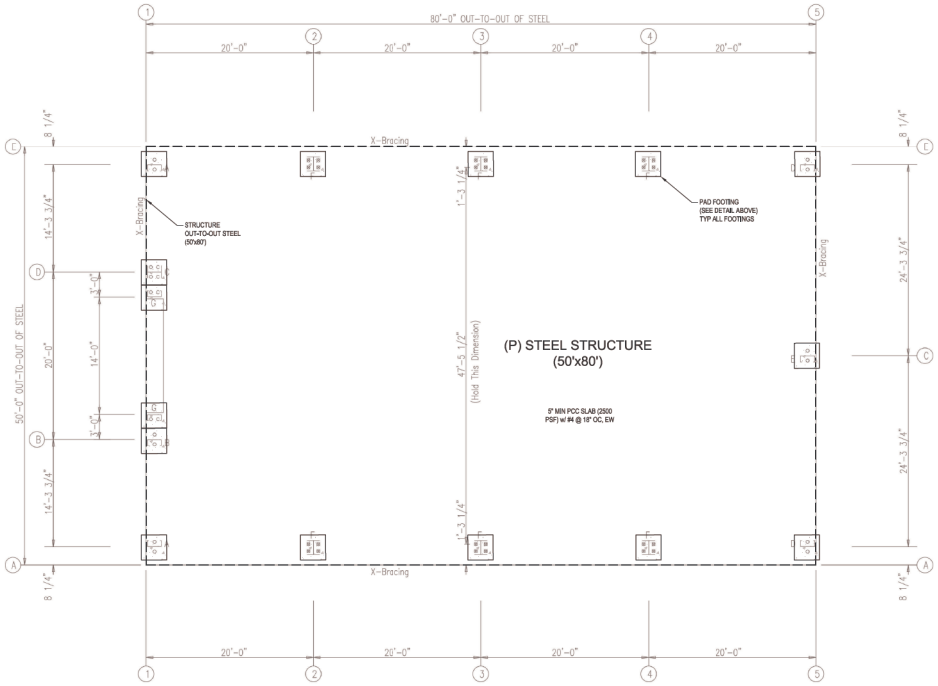
Frm Line	Col Line	Wind Suct Horz	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seis_Left Horz	Seis_Left Vert	Seis_Right Horz	Seis_Right Vert	Seis Long Horz	MIN_SNOW Horz	MIN_SNOW Vert
1	E	0.0	0.0	−0.7	−0.5	−1.6	−2.5	−3.1	0.0	3.8	0.0	0.0	1.6
1	D	2.7	0.5	−3.9	0.0	−1.3	0.0	3.1	2.5	−3.7	0.2	0.0	5.6
1	B	2.7	0.0	−2.0	0.0	−3.1	0.0	0.2	0.0	−0.2	0.2	0.0	5.6
1	A	0.0	0.0	−0.9	0.0	−1.5	0.0	−0.2	0.0	0.2	0.0	0.0	1.6

Frm Line	Col Line	E1UNB_SL_L Horz	E1UNB_SL_L Vert	E1UNB_SL_R Horz	E1UNB_SL_R Vert
1	E	0.0	1.5	0.0	0.3
1	D	0.0	6.6	0.0	2.6
1	B	0.0	2.6	0.0	6.6
1	A	0.0	0.3	0.0	1.5

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind Press Horz
5	A	0.4	0.3	1.9	2.7	0.0	−1.8	0.0	−1.5	0.0	−1.1	0.0	−0.7	0.0
5	C	1.1	1.0	5.9	8.7	−1.8	−6.5	0.0	−3.0	−1.6	−4.8	0.0	−1.7	−4.0
5	E	0.4	0.3	1.9	2.7	0.0	0.5	1.8	−3.4	0.0	1.0	1.6	−2.5	0.0

Frm Line	Col Line	Wind Suct Horz	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seis_Left Horz	Seis_Left Vert	Seis_Right Horz	Seis_Right Vert	Seis Long Horz	MIN_SNOW Horz	MIN_SNOW Vert
5	A	0.0	0.0	−2.3	0.0	−1.2	0.0	0.2	0.0	−0.2	0.0	0.0	2.7
5	C	4.3	0.0	−3.6	−0.5	−4.6	−2.5	−2.7	0.0	2.1	0.3	0.0	8.7
5	E	0.0	0.5	−1.6	0.0	−1.7	0.0	2.6	2.5	−1.9	0.0	0.0	2.7

Frm Line	Col Line	E2UNB_SL_L Horz	E2UNB_SL_L Vert	E2UNB_SL_R Horz	E2UNB_SL_R Vert	E2PAT_LL_1 Horz	E2PAT_LL_1 Vert	E2PAT_LL_2 Horz	E2PAT_LL_2 Vert
5	A	0.0	3.3	0.0	0.4	0.0	2.2	0.0	−0.3
5	C	0.0	7.3	0.0	7.3	0.0	3.0	0.0	3.0
5	E	0.0	0.4	0.0	3.3	0.0	−0.3	0.0	2.2



FOUNDATION PLAN

SCALE: 1" = 7'

811 CALL 811 BEFORE YOU DIG SAFE DIGGING PARTNER

DATE: ~
REVISIONS: 1

PE / PLS:
PROFESSIONAL ENGINEER-STATE OF NEVADA
ANDREW HAMMOND
Exp. 6/30/25
CIVIL
No. 21197
8/28/23

ELEMENT engineering
775.762.5461
andrew@elementengineer.com

A FOUNDATION PLAN FOR THE:
DAVIDSON SHOP
195 E SKY RANCH BLVD
SPARKS, NV 89441 | APN: 534-11 3-02

SCALE: VARIES
DRAWN: AMH
CHECKED: AMH
DATE: 8/28/23
NOTES:

SHEET
FP

BUILDER/CONTRACTOR RESPONSIBILITIES

Drawing Validity – These drawings, supporting structural calculations and design certification are based on the order documents as of the date of these drawings. These documents describe the material supplied by the manufacturer as of the date of these drawings. Any changes to the order documents after the date on these drawings may void these drawings, supporting structural calculations and design certification. The Builder/Contractor is responsible for notifying the building authority of all changes to the order documents which result in changes to the drawings, supporting structural calculations and design certification.

Builder Acceptance of Drawings – Approval of the manufacturer's drawings and design data affirms that the manufacturer has correctly interpreted and applied the requirements of the order documents and constitutes Builder/Contractor acceptance of the manufacturer's interpretations of the order documents and standard product specifications, including its design, fabrication and quality criteria standards and tolerances. (AISC code of standard practice APR 10 Section 4.4.1)

Code Official Approval – It is the responsibility of the Builder/Contractor to ensure that all project plans and specifications comply with the applicable requirements of any governing building authority. The Builder/Contractor is responsible for securing all required approvals and permits from the appropriate agency as required.

Builder is responsible for State, Federal and OSHA safety compliance – The Builder/Contractor is responsible for applying and observing all pertinent safety rules and regulations and OSHA standards as applicable.

Building Erection – The Builder/Contractor is responsible for all erection of the steel and associated work in compliance with the Metal Building Manufacturers drawings. Temporary supports, such as temporary guys, braces, false work or other elements required for erection will be determined, furnished and installed by the erector. (AISC Code of Standard Practice APR 10 Section 7.10.3)

Discrepancies – Where discrepancies exist between the Metal Building plans and plans for other trades, the Metal Building plans will govern. (AISC Code of Standard Practice APR 10 Section 3.3)

Materials by Others – All interface and compatibility of any materials not furnished by the manufacturer are the responsibility of and to be coordinated by the Builder/Contractor or A/E firm. Unless specific design criteria concerning any interface between materials if furnished as a part of the order documents, the manufacturers assumptions will govern.

Modification of the Metal Building from Plans – The Metal Building supplied by the manufacturer has been designed according to the Building Code and specifications and the loads shown on this drawing. Modification of the building configuration, such as removing wall panels or braces, from that shown on these plans could affect the structural integrity of the building. The Metal Building Manufacturer or a Licensed Structural Engineer should be consulted prior to making any changes to the building configuration shown on these drawings. The Metal Building Manufacturer will assume no responsibility for any loads applied to the building not indicated on these drawings.

Foundation Design – The Metal Building Manufacturer is not responsible for the design, materials and workmanship of the foundation. Anchor rod plans prepared by the manufacturer are intended to show only location, diameter and projection of the anchor rods required to attach the Metal Building System to the foundation. It is the responsibility of the end customer to ensure that adequate provisions are made for specifying rod embedment, bearing values, tie rods and or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. (MBMA MBSM Chapter 4 Section 3.2.2 and Section A3)

Shimming – In accordance with Section 6.10 of Chapter 4, Common Industry Practices in the Metal Building Systems Manual, shimming is a normal part of erection and is not subject to claim.

PROJECT NOTES

Material properties of steel bar, plate, and sheet used in the fabrication of built-up structural framing members conform to ASTM A529, ASTM A572, or ASTM A1011 with 55 ksi min. yield, except flanges wider than 12" and thicker than 3/8", all flanges thicker than 1", and all webs thicker than 3/8" are 50 ksi min. yield. Rod X-bracing conforms to ASTM A529 or ASTM A572 with 50 ksi min. yield. Cable X-bracing conforms to ASTM A475 7 Strand Extra High-Strength grade. Hot rolled structural shapes conform to ASTM A992, ASTM A529, or ASTM A572 with 50 ksi min. yield. Hot rolled angles other than flange braces, conform to ASTM A36 minimum. Round and rectangular HSS conforms to ASTM A500 Grade B. Cold-formed steel secondary framing Members conform to ASTM A1011 or ASTM A653 Grade 55 with 55 ksi min. yield.

The manufacturer does not assume any responsibility for the erection nor field supervision of the structure and or any special inspections that may be required by the local building authority during erection (including inspection of the high strength bolts or field welds) as required during erection. The coordination and the costs associated for setting up and Special Inspections are the responsibility of the Erector, Owner, Architect, or Engineer of Record.

Design is based upon the more severe loading of either the roof snow load or the roof ive load.

Loads, as noted, are given within order documents and are applied in general accordance with the applicable provisions of the model code and/or specification indicated. Neither the manufacture nor the certifying engineer declares or attests that the loads as designated are proper for the local provisions that may apply or for site specific parameters. The manufacturer's Engineer's certification is limited to design loads supplied by an Architect and/or engineer of record for the overall construction project.

This project is designed using manufacture's standard serviceability standards. Generally this means that all stresses and deflections are within typical performance limits for normal occupancy and standard metal building products. If special requirements for deflections and vibrations must be adhered to, then they must be clearly stated in the contract documents.

This metal building system is designed as enclosed. All exterior components (i.e. doors, windows, vents, etc.) must be designed to withstand the specified wind loading for the design of components and cladding in accordance with the specified building code. Doors are to be closed when a maximum of 50% of design wind velocity is reached.

Unless otherwise noted, special inspection of fabricated items is not required. Per IBC section 1704.2.5.1, The fabricator is approved to perform such work without special inspection through maintenance of IAS AC 472 certification MB-136

BOLT TIGHTENTING–Boltedjoints withASTMA325Type 1 bolts greaterthan 1/2" diameter are specified as pretensioned joints in accordance with the most recent edition of the RCSC Specification for Structural Joints Using ASTM A325 or A490 Bolts. Pretensioning can be accomplished by using the turn-of-nut method of tightening, calibrated wrench,twist-off-type tension-control bolts or direct-tension-indicator as acceptable to the Inspecting Agency and Building Official. Installation inspection requirements for pretensioned joints (Specification for Structural Joints Section 9.2) using turn-of-nut method is suggested. The connections on this project are not slip critical.

The metal building manufacturer has not designed the structure for snow accumulation loads at the ground level which may impose snow loads on the wall framing provided by the manufacturer.

The design collateral load has been uniformly applied to the design of the building. Hanging loads are to be attached to the purlin web. This may not be appropriate for heavily concentrated loads.Any attached load in excess of 150 pounds shall be accounted for by special design performed by a licensed engineer using concentratedloads and may require separate support members within the roof system.

ENGINEERING DESIGN CRITERIA

Building Code..... IBC 18
Building Risk Category..... II - Normal

Roof Dead Load
Superimposed..... 1.920 psf
Collateral..... 3 psf

Roof Live Load.....20.00 psf Yes reduction

Snow
Ground Snow Load (Pg)..... 40.00 psf
Snow Load Importance Factor (Is) 1.00
Snow Exposure Factor (Ce)..... 1.00
Thermal Factor (Ct)..... 1.00
Flat Roof Snow Load (Pf)..... 28 psf
Minimum Roof Snow Load (Pm)..... 28.00 psf

Wind
Ultimate Wind Speed (Vult)..... 120 mph
Nominal Wind Speed (Vasd)....93 mph (IBC Section 1609.3.1)
Serviceability Wind Speed..... 68 mph
Ground Elevation Factor..... 0.85 (4526.00ft ASL)
Wind Exposure Category..... B
Internal Pressure Coefficient (GCpi) 0.18 / -0.18
Loads for components not provided by building manufacturer.
Wall Edge Zones (within 5.00 ' of corner)
19.18 psf pressure
-25.69 psf suction
Other Wall Zones 19.18 psf pressure
-20.81 psf suction
These values are the maximum values required based on a 10 square foot area.
Components with larger areas may have lower wind loads.
Zones per ASCE 7-16; FIG. 30.3-1
Zones pressures shown are Un-Factored

Seismic
Seismic Importance Factor (Ie)..... 1.00
Seismic Design Category..... D
Soil Site Class..... d
Ss..... 1.416 g Sds..... 1.132 g
S1..... 0.476 g Sd1..... 0.579 g
Analysis Procedure..... Equivalent Lateral Force

Location...	Int RF	Front SW	Back SW	Left EW	Right EW
System.....	C4	B3	B3	B3	B3
R.....	3.25	3.25	3.25	3.25	3.25
Cs.....	0.349	0.349	0.349	0.349	0.349

Design Base Shear in kips (V) Transverse 12.02
Design Base Shear in kips (V) Longitudinal 12.10

Basic Structural System (from ASCE 7-16 Tcble 12.2-1)
System - Basic Force Resisting System
H - Steel System not Specifically Detailed for Seismic Resistance
C4 - Steel Ordinary Moment Frames
B3 - Steel Ordinary Concentric Braced Frames
G2 - Steel Ordinary Cantilevered Column Systems
R - Response Modification Coefficient
Cs - Seismic Response Coefficient
Transverse - Direction Parallel to the Rigid Frames
Longitudinal - Direction Perpendicular to the Rigid Frames

BEJUN ANKLESARIA, PE SE
2918 CREEK TERACE DR.
MISSOURI CITY TX 77459
281-499-1472

Building Descriptions				
Building ID	Width(ft)	Length(ft)	Height(ft)	Slope
Building A	50	80	16	3.0:12

Drawing Index	
Page	Description
C1	COVER SHEET
F1	ANCHOR BOLT PLAN
F2	ANCHOR BOLT REACTIONS
F3	ANCHOR BOLT DETAILS
E1	ROOF FRAMING PLAN
E2	ROOF SHEETING PLAN
E3	FRONT SIDEWALL
E4	BACK SIDEWALL
E5	LEFT ENDWALL
E6	RIGHT ENDWALL
E7	FRAME CROSS SECTION
DET1-24	STANDARD DETAILS
R1-R3	INSTALLATION SHEETS

DRAWING STATUS

☐ FOR APPROVAL
These drawings, being For Approval, are by definition not final, and are for conceptual representation only. Their purpose is to confirm proper interpretation of the project documents. Only drawings issued "For Erector Installation" can be considered as complete.

☐ FOR CONSTRUCTION PERMIT
These drawings, being for Permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered as complete.

☒ FOR ERECTOR INSTALLATION
Final drawings for construction.

For questions or assistance Concerning Erection call:
252-977-2131
Monday-Friday 7:30am to 5:00pm

ENGINEERING SEAL

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

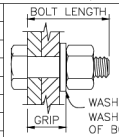
Drawing has been digitally signed.

PROFESSIONAL ENGINEER-STATE OF NEVADA
BEJUN N. ANKLESARIA
Exp: 12/31/24
STRUCTURAL
Feb 07, 2023
No. 17245

Download panel installation manuals from:
www.cornerstonebuildingbrands.com/installationmanuals/

Descargue los manuales de instalación del panel desde:
www.cornerstonebuildingbrands.com/installationmanuals/

1/2"ø A325 BOLT GRIP TABLE (UNLESS NOTED)	
GRIP	LENGTH
0 TO 9/16"	1 1/4" F.T.
Over 9/16" TO 1 1/16"	1 3/4" F.T.
Over 1 1/16" TO 1 5/16"	2"
Over 1 5/16" TO 1 9/16"	2 1/4"
Over 1 9/16" TO 1 13/16"	2 1/2"
Over 1 13/16" TO 2 1/16"	2 3/4"
LOCATIONS OF BOLTS LONGER THAN 2 3/4" NOTED ON ERECTION DRAWINGS	
F.T. DENOTES FULLY THREADED	



NOTE:
FULL THREAD ENGAGEMENT IS DEEMED TO HAVE BEEN MET WHEN THE END OF THE BOLT IS FLUSH WITH THE FACE OF THE NUT.

WASHER REQUIRED ONLY WHEN SPECIFIED. WASHER MAY BE LOCATED UNDER HEAD OF BOLT, UNDER NUT, OR AT BOTH AT LOCATIONS NOTED ON ERECTION DRAWINGS. ADD 5/32" FOR EACH WASHER TO MATERIAL THICKNESS TO DETERMINE GRIP.

Rev. 9/29/2022

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	2/ 3/23	FOR ERECTOR INSTALLATION	IES	IES	SXV

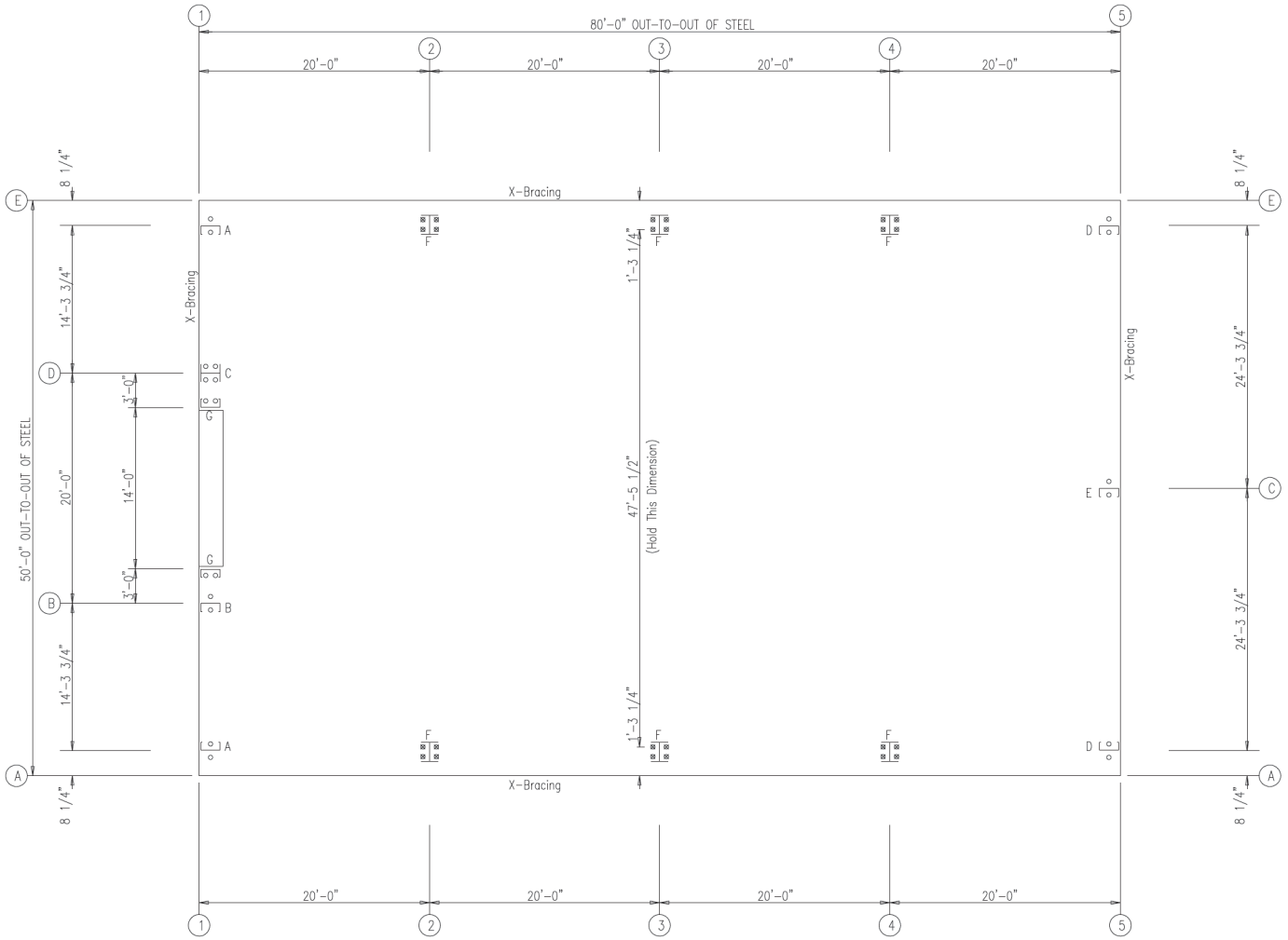
CECO Building Systems
Columbus, MS. (662) 328-6722
Mount Pleasant, IA. (319) 385-8001
Rocky Mount, NC. (252) 977-2131
www.cecobuildings.com

PROJECT: NATALIE DAVIDSON
CUSTOMER: THE STEEL BUILDER OWNER: NATALIE DAVIDSON
LOCATION: SPARKS,NV 89441-8549 US

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	2/ 3/23	N.T.S.	1	A	19-B-27926	C1	0

○ Dia= 5/8"

⊠ Dia= 3/4"



ANCHOR BOLT PLAN

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	2/ 3/23	FOR ERECTOR INSTALLATION	IES	IES	SXV

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Exp: 12/31/24
STRUCTURAL
No. 17245
Feb 07, 2023

GENERAL NOTES

1) THE REACTIONS PROVIDED ARE BASED ON THE ORDER DOCUMENTS AT THE TIME OF MAILING. ANY CHANGES TO BUILDING LOADS OR DIMENSIONS MAY CHANGE THE REACTIONS. THE REACTIONS WILL BE SUPERSEDED AND VOIDED BY ANY FUTURE MAILING.
2) THE REACTIONS PROVIDED HAVE BEEN CREATED WITH THE FOLLOWING LAYOUT (UNLESS NOTHED OTHERWISE)
   A) A REACTION TABLE IS PROVIDED WITH REACTIONS FOR EACH LOAD GROUP
   B) RIGID FRAMES
      (1) SEE NOTE 3.
   C) ENDWALLS
      (1) SEE NOTE 3.
   D) X-BRACING
      1) X-BRACING REACTIONS ARE INCLUDED IN VALUES SHOWN IN THE REACTION TABLES AS NOTED IN THE BRACING REACTIONS TABLE.
      2) FOR IBC AND UBC BASED BUILDING CODES, WHEN X-BRACING IS PRESENT IN THE SIDEWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS DO NOT INCLUDE THE AMPLIFICTION FACTOR, OMEGAo.
      3) FOR IBC AND UBC BASED BUILDING CODES, WHEN X-BRACING IS PRESENT IN THE ENDWALL, INDIVIDUAL TRANSVERSE SEISMIC LOADS DO NOT INCLUDE THE AMPLIFICATION FACTOR, OMEGAo.
   E) THE METAL BUILDING MANUFACTURER IS RESPONSIBLE ONLY FOR THE PORTION OF THE ANCHOR ROD DESIGN PERTAINING TO THE TRANSFER OF FORCES BETWEEN THE BASE PLATE BEARING AND THE ANCHOR ROD?S SHEAR AND TENSION. THE METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR THE ANCHOR ROD EMBEDMENT FOR TRANSFER OF FORCES TO THE FOUNDATION. THE METAL BUILDING MANUFACTURER DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL, AND CONSTRUCTION OF THE FOUNDATION EMBEDMENT. THE END USE CUSTOMER SHALL ASSURE THAT ADEQUATE PROVISIONS ARE MADE TO THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE AND FOUNDATION OF THE BUILDING BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER COMPETENT IN THE DESIGN OF SUCH STRUCTURES.
      i) (REF. APPENDIX A3 OF THE MBMA METAL BUILDING BUILDING SYSTEMS MANUAL)
   F) ANCHOR RODS ARE ASTM F1554 GR. 36 MATERIAL UNLESS NOTED OTHERWISE ON THE ANCHOR ROD LAYOUT DRAWING.
3) REACTIONS ARE PROVIDED AS UN-FACTORED FOR EACH LOAD GROUP APPLIED TO THE COLUMN. THE FACTORS APPLIED TO LOAD GROUPS FOR THE STEEL COLUMN DESIGN MAY BE DIFFERENT THAN THE FACTORS USED IN THE FOUNDATION DESIGN. THE FOUNDATION ENGINEER SHALL APPLY THE APPROPRIATE LOAD FACTORS AND COMBINE THE REACTIONS IN ACCORDANCE WITH THE BUILDING CODE AND DESIGN SPECIFICATIONS FOR PROPER FOUNDATION DESIGN.
   A) FOR PROJECTS USING ULTIMATE DESIGN WIND SPEEDS SUCH AS 2012 IBC, 2015 IBC, OR FLORIDA BUILDING CODE, THE WIND LOAD REACTIONS ARE AT A STRENGTH VALUE WITH A LOAD FACTOR OF 1.0.
   B) FOR IBC CODES, THE SEISMIC REACTIONS PROVIDED ARE AT A STRENGTH LEVEL WITH A LOAD FACTOR OF 1.0, AND DO NOT CONTAIN THE RHO FACTOR.

THE MANUFACTURER DOES NOT PROVIDE "MAXIMUM" LOAD COMBINATION REACTIONS. HOWEVER, THE INDIVIDUAL LOAD REACTIONS PROVIDED MAY BE USED BY THE FOUNDATION ENGINEER TO DETERMINE THE APPLICABLE LOAD COMBINATIONS FOR HIS/HER DESIGN PROCEDURES AND ALLOW FOR AN ECONOMICAL FOUNDATION DESIGN.

BUILDING BRACING REACTIONS

Wall Loc	Wall Line	Col Line	Reactions in plane of wall ± Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel_Shear (lb/ft) Wind	Seis
L_EW	1	E,D	Bracing, see EW reactions					
F_SW	A	2,3	3.8	*	7.9	*		
R_EW	5	C,E	Bracing, see EW reactions					
B_SW	E	3,2	3.8	*	7.9	*		

*See RF reactions table for vertical and horizontal reactions in plane of the rigid frame.

FLOOR REACTION NOTE:

FOR VERTICAL LOADS, UPWARD IS POSITIVE AND DOWNWARD IS NEGATIVE. FOR HORIZONTAL REACTIONS, TO THE RIGHT IS POSITIVE AND TO THE LEFT IS NEGATIVE. FOUNDATION LOADS ARE IN OPPOSITE DIRECTIONS.

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind Press Horz
1	E	0.2	0.2	1.1	1.6	−1.8	−3.4	0.0	1.7	−1.6	−2.5	0.0	1.9	0.0
1	D	0.7	0.6	4.0	5.6	0.0	−1.3	1.8	−5.0	0.0	−0.7	1.6	−3.7	−2.5
1	B	0.6	0.6	4.0	5.6	0.0	−2.1	0.0	−3.7	0.0	−1.2	0.0	−2.8	−2.5
1	A	0.2	0.2	1.1	1.6	0.0	−1.2	0.0	−0.9	0.0	−0.6	0.0	−0.4	0.0

Frm Line	Col Line	Wind Suct Horz	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seis_Left Horz	Seis_Left Vert	Seis_Right Horz	Seis_Right Vert	Seis Long Horz	−MIN_SNOW− Horz	−MIN_SNOW− Vert
1	E	0.0	0.0	−0.7	−0.5	−1.6	−2.5	−3.1	0.0	3.8	0.0	0.0	1.6
1	D	2.7	0.5	−3.9	0.0	−1.3	0.0	3.1	2.5	−3.7	0.2	0.0	5.6
1	B	2.7	0.0	−2.0	0.0	−3.1	0.0	0.2	0.0	−0.2	0.2	0.0	5.6
1	A	0.0	0.0	−0.9	0.0	−1.5	0.0	−0.2	0.0	0.2	0.0	0.0	1.6

Frm Line	Col Line	E1UNB_SL_L− Horz	E1UNB_SL_L− Vert	E1UNB_SL_R− Horz	E1UNB_SL_R− Vert
1	E	0.0	1.5	0.0	0.3
1	D	0.0	6.6	0.0	2.6
1	B	0.0	2.6	0.0	6.6
1	A	0.0	0.3	0.0	1.5

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind Press Horz
5	A	0.4	0.3	1.9	2.7	0.0	−1.8	0.0	−1.5	0.0	−1.1	0.0	−0.7	0.0
5	C	1.1	1.0	5.9	8.7	−1.8	−6.5	0.0	−3.0	−1.6	−4.8	0.0	−1.7	−4.0
5	E	0.4	0.3	1.9	2.7	0.0	0.5	1.8	−3.4	0.0	1.0	1.6	−2.5	0.0

Frm Line	Col Line	Wind Suct Horz	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seis_Left Horz	Seis_Left Vert	Seis_Right Horz	Seis_Right Vert	Seis Long Horz	−MIN_SNOW− Horz	−MIN_SNOW− Vert
5	A	0.0	0.0	−2.3	0.0	−1.2	0.0	0.2	0.0	−0.2	0.0	0.0	2.7
5	C	4.3	0.0	−3.6	−0.5	−4.6	−2.5	−2.7	0.0	2.1	0.3	0.0	8.7
5	E	0.0	0.5	−1.6	0.0	−1.7	0.0	2.6	2.5	−1.9	0.0	0.0	2.7

Frm Line	Col Line	E2UNB_SL_L− Horz	E2UNB_SL_L− Vert	E2UNB_SL_R− Horz	E2UNB_SL_R− Vert	E2PAT_LL_1− Horz	E2PAT_LL_1− Vert	E2PAT_LL_2− Horz	E2PAT_LL_2− Vert
5	A	0.0	3.3	0.0	0.4	0.0	2.2	0.0	−0.3
5	C	0.0	7.3	0.0	7.3	0.0	3.0	0.0	3.0
5	E	0.0	0.4	0.0	3.3	0.0	−0.3	0.0	2.2

NOTES FOR REACTIONS

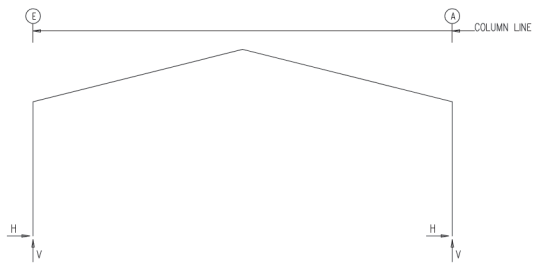
BUILDING REACTIONS ARE BASED ON THE FOLLOWING BUILDING DATA:

WIDTH (FT)	= 50
LENGTH (FT)	= 80
EAVE HEIGHT (FT)	= 16 / 16
ROOF SLOPE (rise/12)	= 3.0:12 / 3.0:12
DEAD LOAD (psf)	= 1.920
COLLATERAL LOAD (psf)	= 3
ROOF LIVE LOAD (psf)	= 20.00 (Reducible)
FRAME LIVE LOAD (psf)	= 12
FLAT ROOF SNOW LOAD (psf)	= 28
GROUND SNOW LOAD (psf)	= 40.00
ULTIMATE WIND SPEED (VULT)	= 120 mph
NOMINAL WIND SPEED (VASD)	= 93 mph (IBC Section 1609.3.1)
SERVICEABILITY WIND SPEED	= 68 mph
WIND CODE	= IBC 18
EXPOSURE	= B
CLOSED/OPEN	= Closed
IMPORTANCE − WIND	= 1.00
IMPORTANCE − SEISMIC	= 1.00
SEISMIC ZONE	= D

REACTION KEY:

WIND Left/Right 1 = (with +GCpi Internal Pressure)
WIND Left/Right 2 = (with −GCpi Internal Pressure)
Wind_Long 1 = Wind Load Case B at Left EW
Wind_Long 2 = Wind Load Case B at Right EW
MIN_SNOW = Minimum Snow (Pm) per code
E1UNB_SL_L = Endwall Unbalanced Snow Left
E1UNB_SL_R = Endwall Unbalanced Snow Right
F1UNB_SL_L = Rigid Frame Unbalanced Snow Left
F1UNB_SL_R = Rigid Frame Unbalanced Snow Right

FRAME LINES: 2 3 4



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc._Bolt Qty	Anc._Bolt Dia	Base_Plate (in) Width	Base_Plate (in) Length	Thick	Grout (in)
2*	E	4	0.750	6.000	10.50	0.375	0.0
2*	A	4	0.750	6.000	10.50	0.375	0.0

2* Frame lines: 2 3 4

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	−Dead− Horiz	−Dead− Vert	−Collateral− Horiz	−Collateral− Vert	−Live− Horiz	−Live− Vert	−Snow− Horiz	−Snow− Vert	−Wind_Left1− Horiz	−Wind_Left1− Vert	−Wind_Right1− Horiz	−Wind_Right1− Vert
2*	E	0.6	1.6	0.7	1.5	2.9	6.0	6.7	14.0	−5.7	−8.4	0.5	−5.4
2*	A	−0.6	1.6	−0.7	1.5	−2.9	6.0	−6.7	14.0	−0.5	−5.4	5.7	−8.4

Frame Line	Column Line	−Wind_Left2− Horiz	−Wind_Left2− Vert	−Wind_Right2− Horiz	−Wind_Right2− Vert	−Wind_Long1− Horiz	−Wind_Long1− Vert	−Wind_Long2− Horiz	−Wind_Long2− Vert	−Seismic_Left Horiz	−Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
2*	E	−5.7	−5.1	0.4	−2.0	−0.6	−9.8	−1.3	−8.8	−1.8	−1.1	1.8	1.1
2*	A	−0.4	−2.0	5.7	−5.1	1.3	−8.8	0.6	−9.8	−1.8	1.1	1.8	−1.1

Frame Line	Column Line	−Seismic_Long Horiz	−Seismic_Long Vert	−MIN_SNOW− Horiz	−MIN_SNOW− Vert	F1UNB_SL_L− Horiz	F1UNB_SL_L− Vert	F1UNB_SL_R− Horiz	F1UNB_SL_R− Vert
2*	E	0.0	−5.6	6.7	14.0	5.6	13.6	5.6	8.0
2*	A	0.0	−5.6	−6.7	14.0	−5.6	8.0	−5.6	13.6

2* Frame lines: 2 3 4

ENDWALL COLUMN: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc._Bolt Qty	Anc._Bolt Dia	Base_Plate (in) Width	Base_Plate (in) Length	Thick	Grout (in)
1	E	2	0.625	7.000	8.000	0.250	0.0
1	D	4	0.625	6.000	8.000	0.375	0.0
1	B	2	0.625	7.000	8.000	0.250	0.0
1	A	2	0.625	7.000	8.000	0.250	0.0
5	A	2	0.625	7.000	12.00	0.250	0.0
5	C	2	0.625	7.000	12.00	0.250	0.0
5	E	2	0.625	7.000	12.00	0.250	0.0

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Proj (in)
○ 4	Jamb	5/8"	F1554	2.00
○ 16	Endwall	5/8"	F1554	2.00
⊠ 24	Frame	3/4"	F1554	2.50

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PROJECT: NATALIE DAVIDSON
CUSTOMER: THE STEEL BUILDER
OWNER: NATALIE DAVIDSON
LOCATION: SPARKS,NV 89441-8549 US

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	2/ 3/23	N.T.S.	1	A	19-B-27926	F2	0

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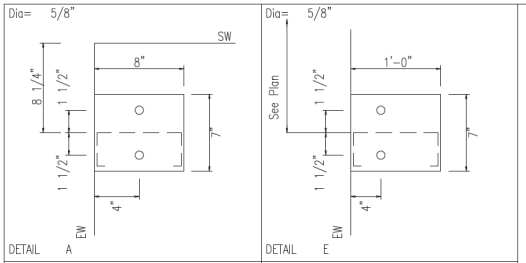

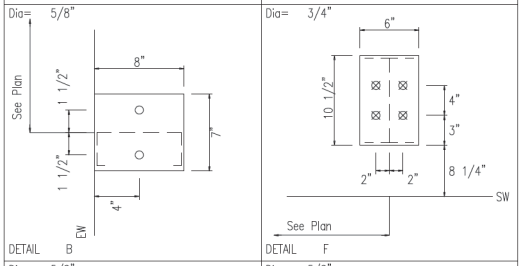

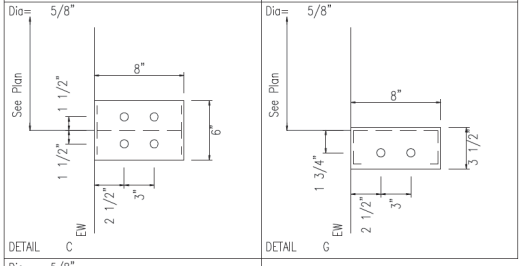

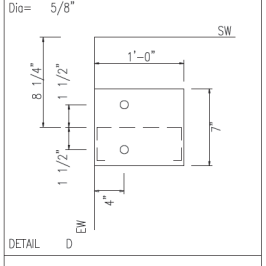

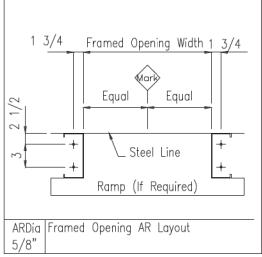

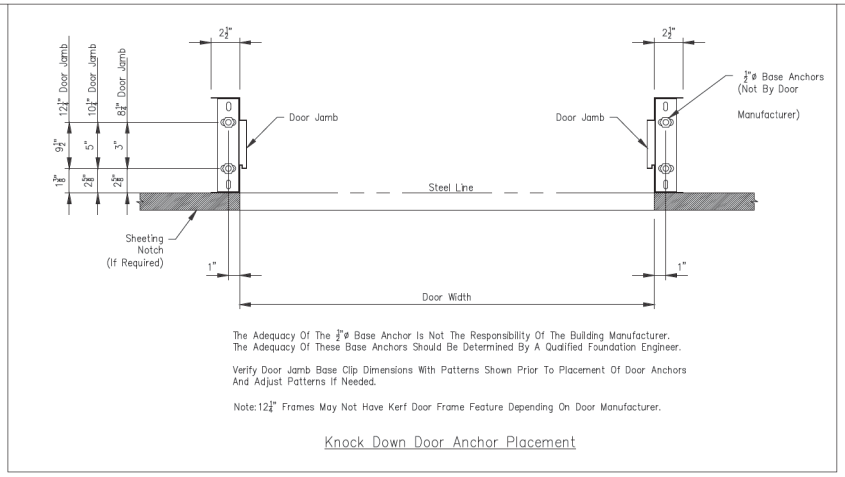

The Adequacy Of The ½"ø Base Anchor Is Not The Responsibility Of The Building Manufacturer. The Adequacy Of These Base Anchors Should Be Determined By A Qualified Foundation Engineer.

Verify Door Jamb Base Clip Dimensions With Patterns Shown Prior To Placement Of Door Anchors And Adjust Patterns If Needed.

Note: 12 3/4" Frames May Not Have Kerf Door Frame Feature Depending On Door Manufacturer.

Knock Down Door Anchor Placement

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PROJECT:	NATALIE DAVIDSON						
CUSTOMER:	THE STEEL BUILDER			OWNER:	NATALIE DAVIDSON		
LOCATION:	SPARKS,NV 89441-8549 US						
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
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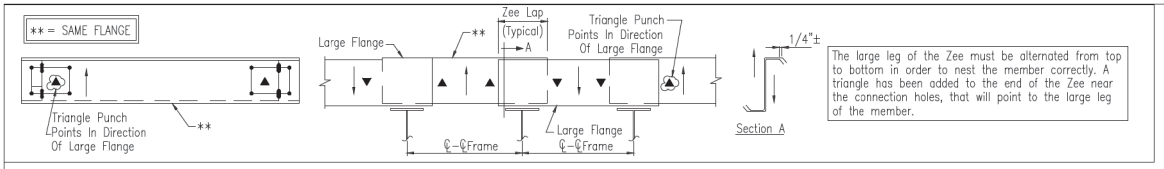


The large leg of the Zee must be alternated from top to bottom in order to nest the member correctly. A triangle has been added to the end of the Zee near the connection holes, that will point to the large leg of the member.

SPECIAL BOLTS
ROOF PLAN

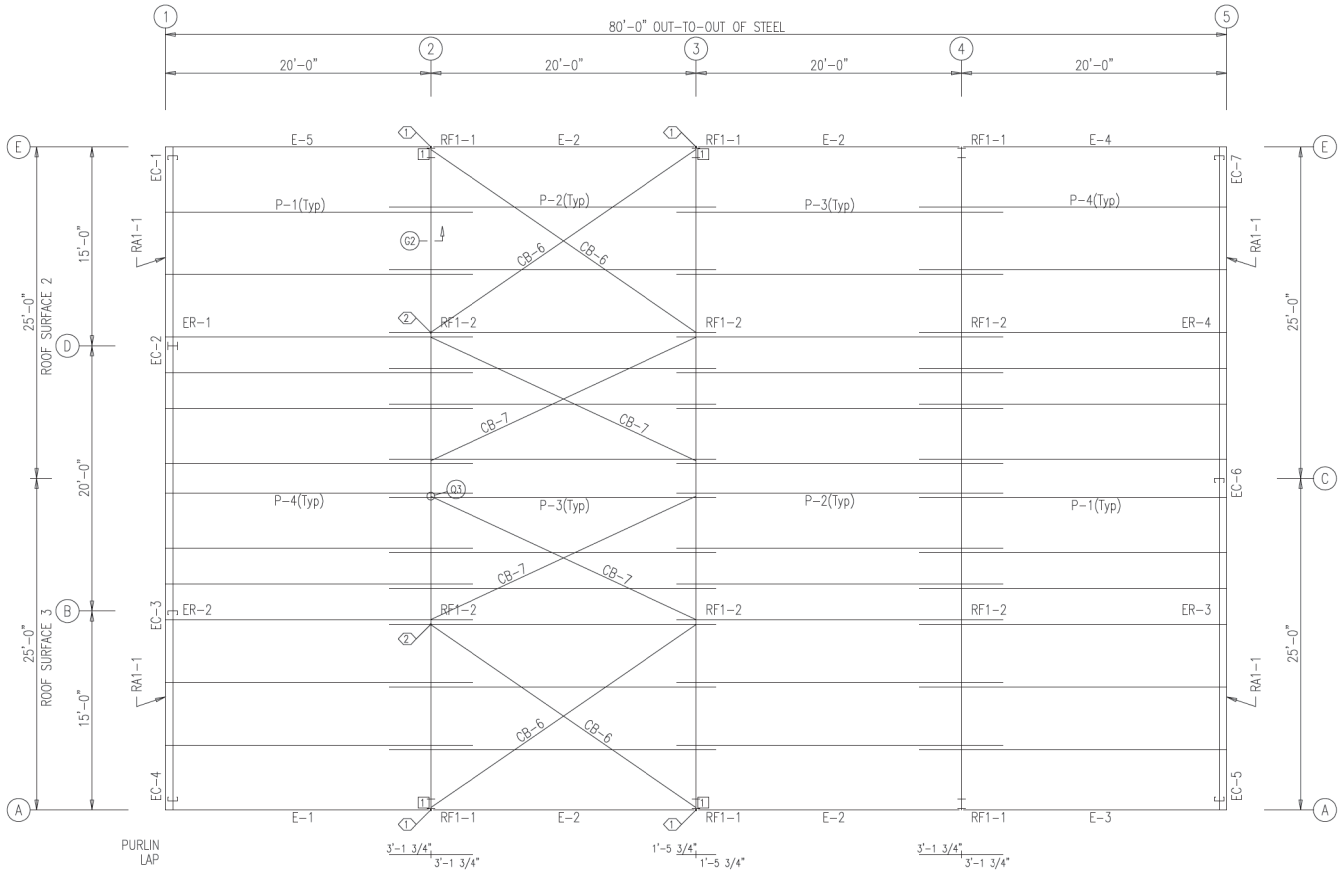
ID	QUAN	TYPE	DIA	LENGTH	WASH
1	4	A325	1/2"	1 1/4"	0
2	2	A325	1/2"	1 1/4"	2

MEMBER TABLE
ROOF PLAN

MARK	PART	LENGTH
P-1	8X25Z16	23'-1 1/2"
P-2	8X25Z16	24'-7 1/2"
P-3	8X25Z16	24'-7 1/2"
P-4	8X25Z16	23'-1 1/2"
E-1	8ES3L14	19'-11 1/2"
E-2	8ES3L14	19'-11 1/2"
E-3	8ES3L14	19'-11 1/2"
E-4	8ES3L14	19'-11 1/2"
E-5	8ES3L14	19'-11 1/2"
CB-6	5/8" DIA. ROD	24'-6"
CB-7	1/2" DIA. ROD	22'-8"

CONNECTION PLATES
ROOF PLAN

ID	MARK/PART
1	SC18



ROOF FRAMING PLAN

GENERAL NOTES:

1. INSTALL ALL PURLIN AND FLANGE BRACES (FB) AS SHOWN.
2. ROOF PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. STRUT PURLINS, IF PROVIDED, MUST BE INSTALLED AND FASTENED TO ROOF SHEETING PER "PBR" PANEL ROOF DETAIL.
4. DO NOT ADD ANY ADDITIONAL ROOF OPENINGS WITHOUT BUILDING MANUFACTURER APPROVAL OR PROFESSIONAL ENGINEER APPROVAL.
5. DO NOT STACK SHEET BUNDLES ON ROOF. ONLY RAISE INDIVIDUAL SHEETS AS NEEDED.
6. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

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Columbus, MS. (662) 328-6722
Mount Pleasant, IA. (319) 385-8001
Rocky Mount, NC. (252) 977-2131
www.cecobuildings.com

PROJECT: NATALIE DAVIDSON
CUSTOMER: THE STEEL BUILDER
OWNER: NATALIE DAVIDSON
LOCATION: SPARKS,NV 89441-8549 US

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	2/ 3/23	N.T.S.	1	A	19-B-27926	E1	0

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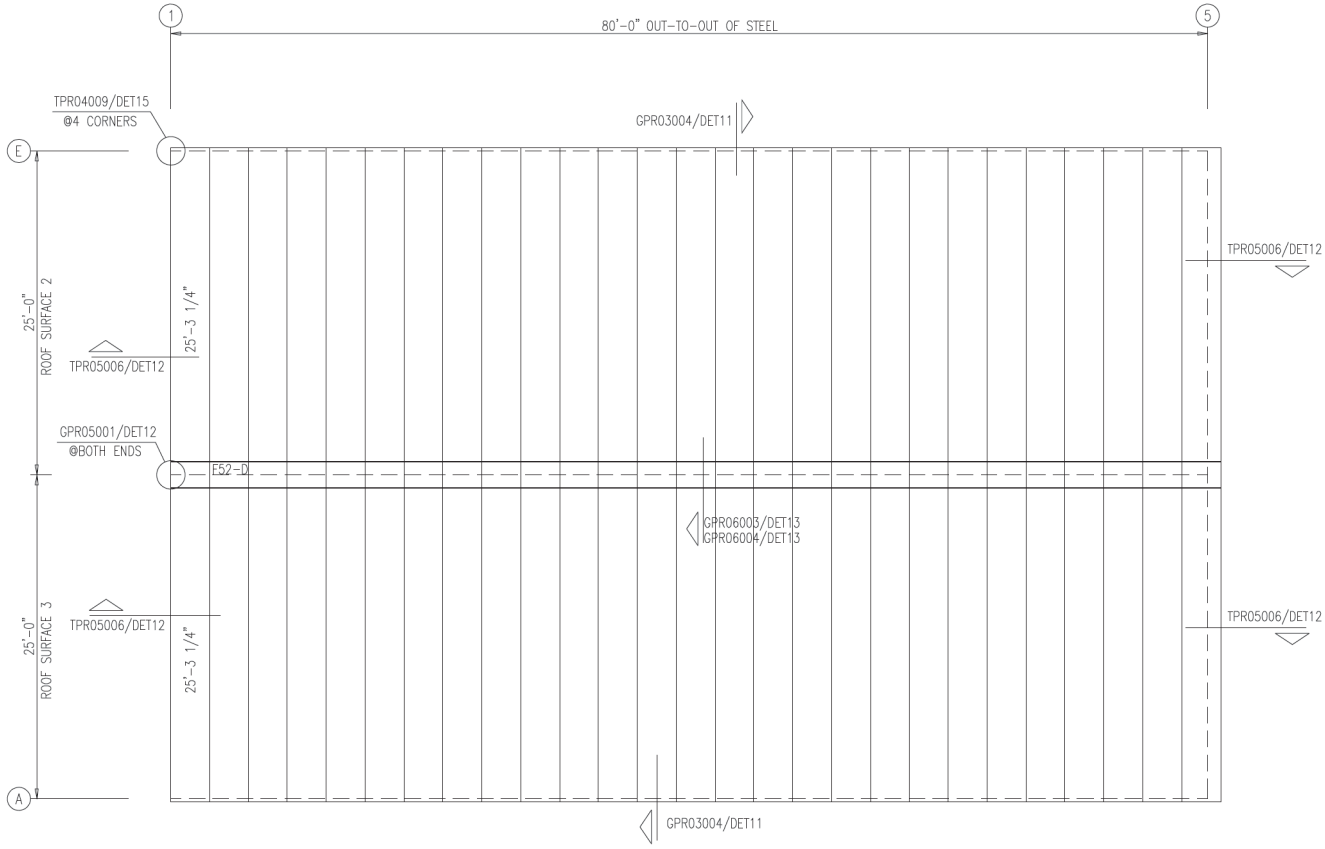
PBR ROOF SHEETING NOTE:
PBR ROOF PANELS ARE TO BE FIELD CUT IF THE PANELS EXTEND OUTSIDE OF THE ROOF PLANE, PANELS ARE NOT TO BE BACK LAPPED.



NOTE: (8 QTY)
LTP'S TO BE FIELD LOCATED & FIELD CUT (BY OTHERS)

INSTALLATION MUST COMPLY WITH OSHA REQUIREMENTS

ROOF SHEETING PLAN

PANELS: 26 Gauge PBR – Galvalume

1) 8 QTY OF LTP'S ARE FIELD SEE DETAIL GPR25100/DET17
2) 1 QTY OF RIDGE VENT IS FIELD SEE DETAIL GPR13202/DET16

GENERAL NOTES:

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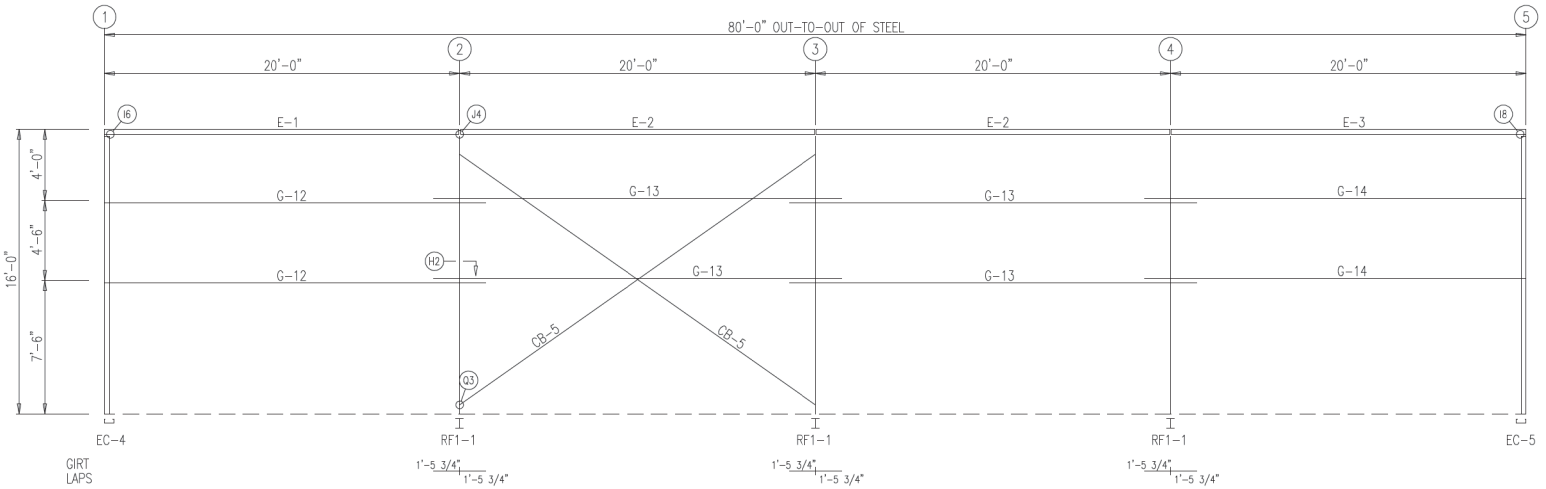
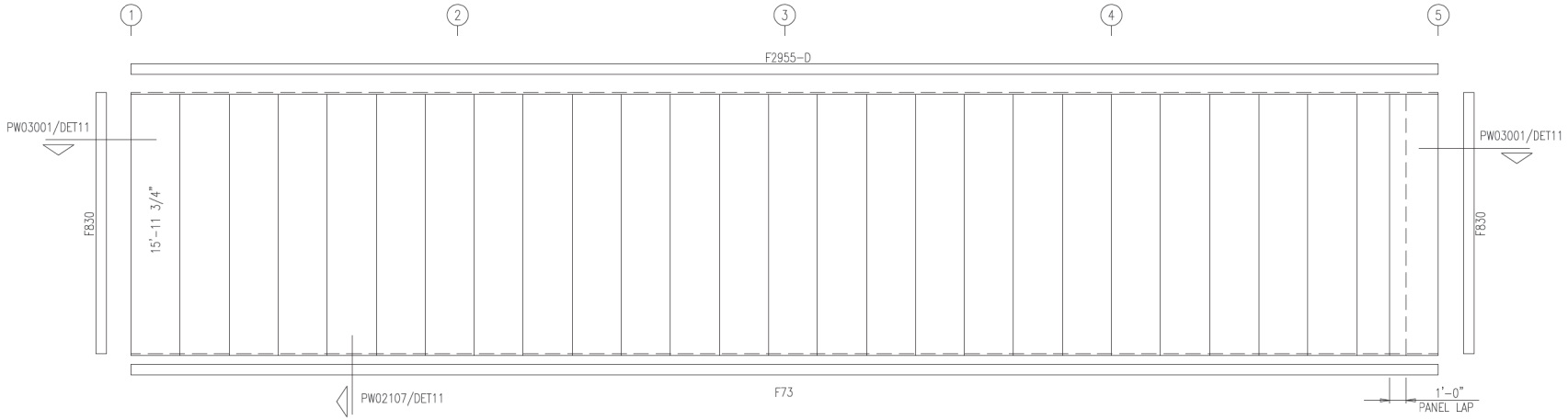
No. 17245
Feb 07, 2023

MEMBER TABLE FRAME LINE A		
MARK	PART	LENGTH
E-1	8ES3L14	19'-11 1/2"
E-2	8ES3L14	19'-11 1/2"
E-3	8ES3L14	19'-11 1/2"
G-12	8X25Z16	21'-5 1/2"
G-13	8X25Z16	22'-11 1/2"
G-14	8X25Z16	21'-5 1/2"
CB-5	3/4" DIA. ROD	25'-4"



SIDEWALL FRAMING: FRAME LINE A



SIDEWALL SHEETING & TRIM: FRAME LINE A

PANELS: 26 Gauge PBR – Polar White

GENERAL NOTES:

1. INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.
2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.
4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

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CUSTOMER:	THE STEEL BUILDER	OWNER:	NATALIE DAVIDSON
LOCATION:	SPARKS,NV 89441-8549 US		

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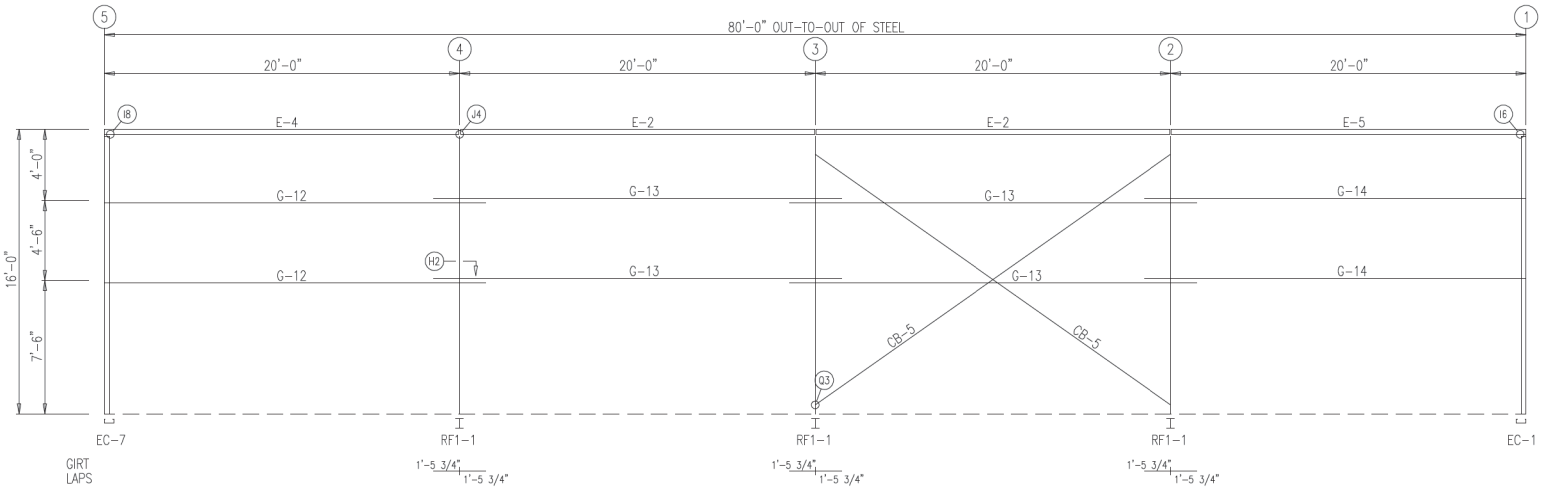
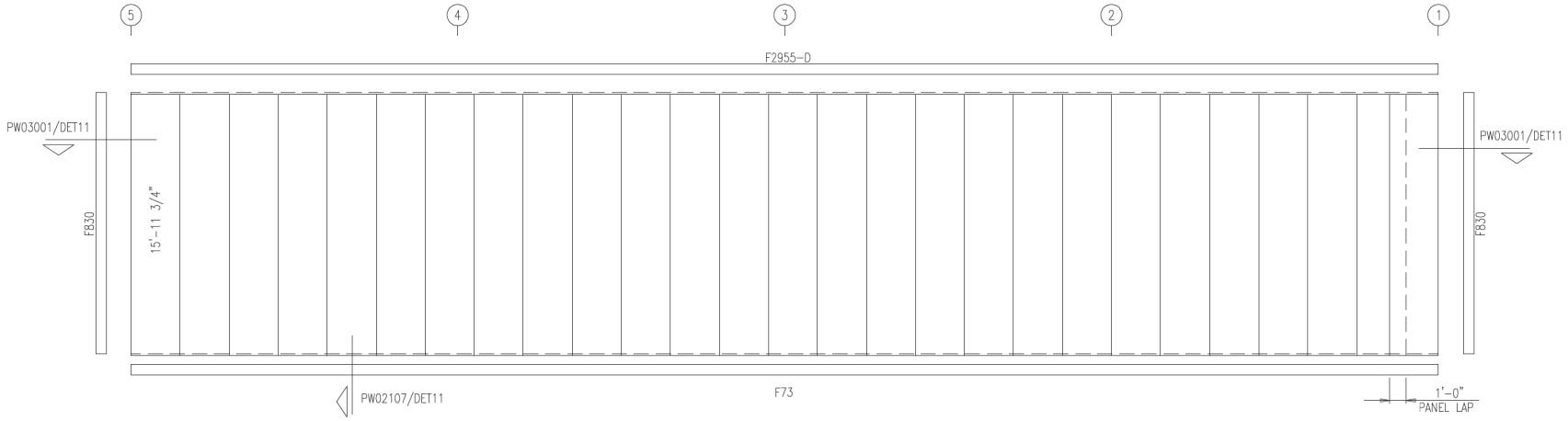
MEMBER TABLE FRAME LINE E		
MARK	PART	LENGTH
E-2	8ES3L14	19'-11 1/2"
E-4	8ES3L14	19'-11 1/2"
E-5	8ES3L14	19'-11 1/2"
G-12	8X25Z16	21'-5 1/2"
G-13	8X25Z16	22'-11 1/2"
G-14	8X25Z16	21'-5 1/2"
CB-5	3/4" DIA. ROD	25'-4"



SIDEWALL FRAMING: FRAME LINE E



SIDEWALL SHEETING & TRIM: FRAME LINE E

PANELS: 26 Gauge PBR – Polar White

GENERAL NOTES:

1. INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.
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CUSTOMER:	THE STEEL BUILDER			OWNER:	NATALIE DAVIDSON		
LOCATION:	SPARKS,NV 89441-8549 US						
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Feb 07, 2023
No. 17245

BEARING FRAME ONLY!

WASHER TO BE USED AT ENDWALL COLUMN TO ENDWALL RAFTER CONNECTION. USE ONE WASHER ON COLUMN SIDE. WASHER NOT NEEDED ON CLIP SIDE.

BOLT TABLE
FRAME LINE 1

LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-2	8	A325	5/8"	1 3/4"
Columns/Raf	4	A325	1/2"	1 1/4"

MEMBER TABLE
FRAME LINE 1

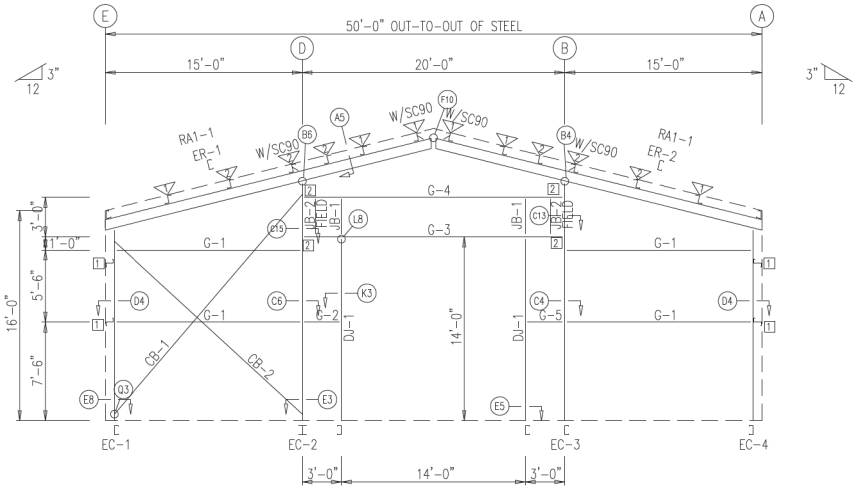
MARK	PART	LENGTH
EC-1	8F35C13	14'-6 1/4"
EC-2	W8X10	18'-1 3/16"
EC-3	8F25C12	18'-1 3/16"
EC-4	8F25C14	14'-6 1/4"
ER-1	10F35C12	25'-9"
ER-2	10F35C12	25'-9"
DJ-1	8X35C14	14'-0"
JB-1	8X35C14	3'-0"
JB-2	8X35C14	2'-4"
G-1	8X25Z16	13'-7 3/4"
G-2	8X25Z16	2'-4 1/4"
G-3	8X35C14	19'-7 3/4"
G-4	8X25Z16	19'-7 3/4"
G-5	8X25Z16	2'-8"
CB-1	1/2" DIA. ROD	22'-8"
CB-2	1/2" DIA. ROD	20'-1"

FLANGE BRACE TABLE
FRAME LINE 1

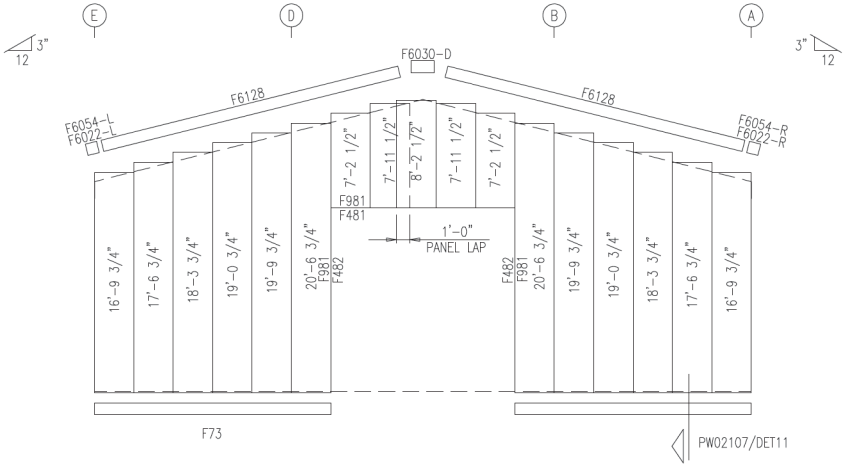
MARK	▽ ID	PART	LENGTH
1	FB30	L2X2X14G	2'-6"
2	FB6-1	L2X2X1/8	2'-6"

CONNECTION PLATES
FRAME LINE 1

□ID	MARK/PART
1	SC5
2	CL751



ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Gauge PBR – Polar White

REFER DETAIL CF02035 FOR SC90 PLATE

GENERAL NOTES:

1. INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.
2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
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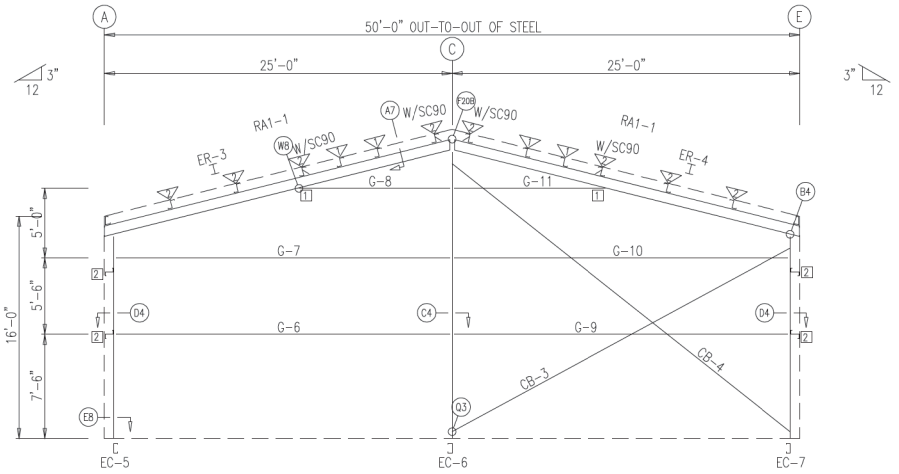
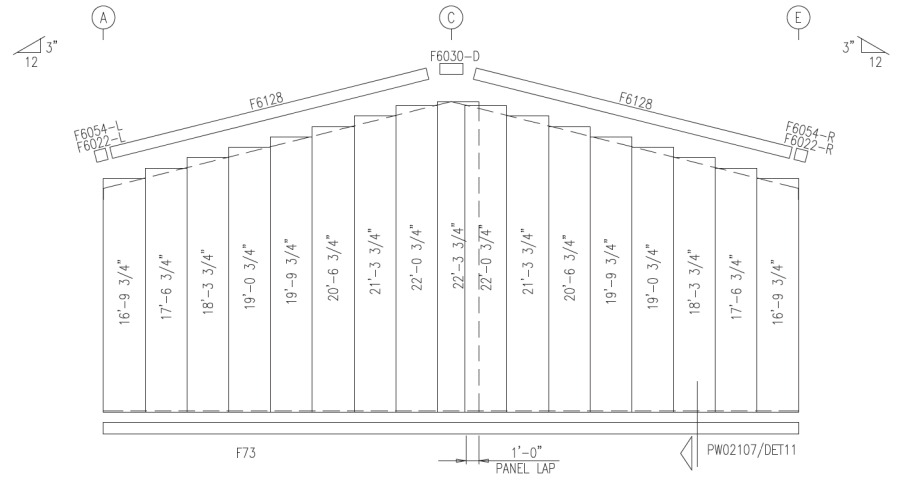


ENDWALL FRAMING: FRAME LINE 5



ENDWALL SHEETING & TRIM: FRAME LINE 5

PANELS: 26 Gauge PBR – Polar White

BEARING FRAME ONLY!

WASHER TO BE USED AT ENDWALL COLUMN TO ENDWALL RAFTER CONNECTION. USE ONE WASHER ON COLUMN SIDE. WASHER NOT NEEDED ON CLIP SIDE.

BOLT TABLE
FRAME LINE 5

LOCATION	QUAN	TYPE	DIA	LENGTH
ER-3/ER-4	8	A325	5/8"	2"
Cor_Column/Raf	4	A325	1/2"	1 1/4"
EC-6/ER-3	4	A325	5/8"	1 1/4"

MEMBER TABLE
FRAME LINE 5

MARK	PART	LENGTH
EC-5	12F25C14	14'-6 3/8"
EC-6	12F35C12	20'-4 3/4"
EC-7	12F35C14	14'-6 3/8"
ER-3	W10X12	25'-9"
ER-4	W10X12	25'-9"
G-6	8X35Z13	23'-7 3/4"
G-7	8X35Z14	23'-7 3/4"
G-8	8X25Z16	9'-1 3/4"
G-9	8X35Z13	23'-11 1/2"
G-10	8X35Z14	23'-11 1/2"
G-11	8X25Z16	9'-1 3/4"
CB-3	1/2" DIA. ROD	28'-4"
CB-4	1/2" DIA. ROD	31'-7"

FLANGE BRACE TABLE
FRAME LINE 5

MARK	▽ ID	PART	LENGTH
1	FB29.8	L2X2X14G	2'-5 3/4"
2	FB6-2	L2X2X1/8	2'-5 3/4"

CONNECTION PLATES
FRAME LINE 5

ID	MARK/PART
1	CL549
2	SC5

REFER DETAIL CF02035 FOR SC90 PLATE

GENERAL NOTES:

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2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
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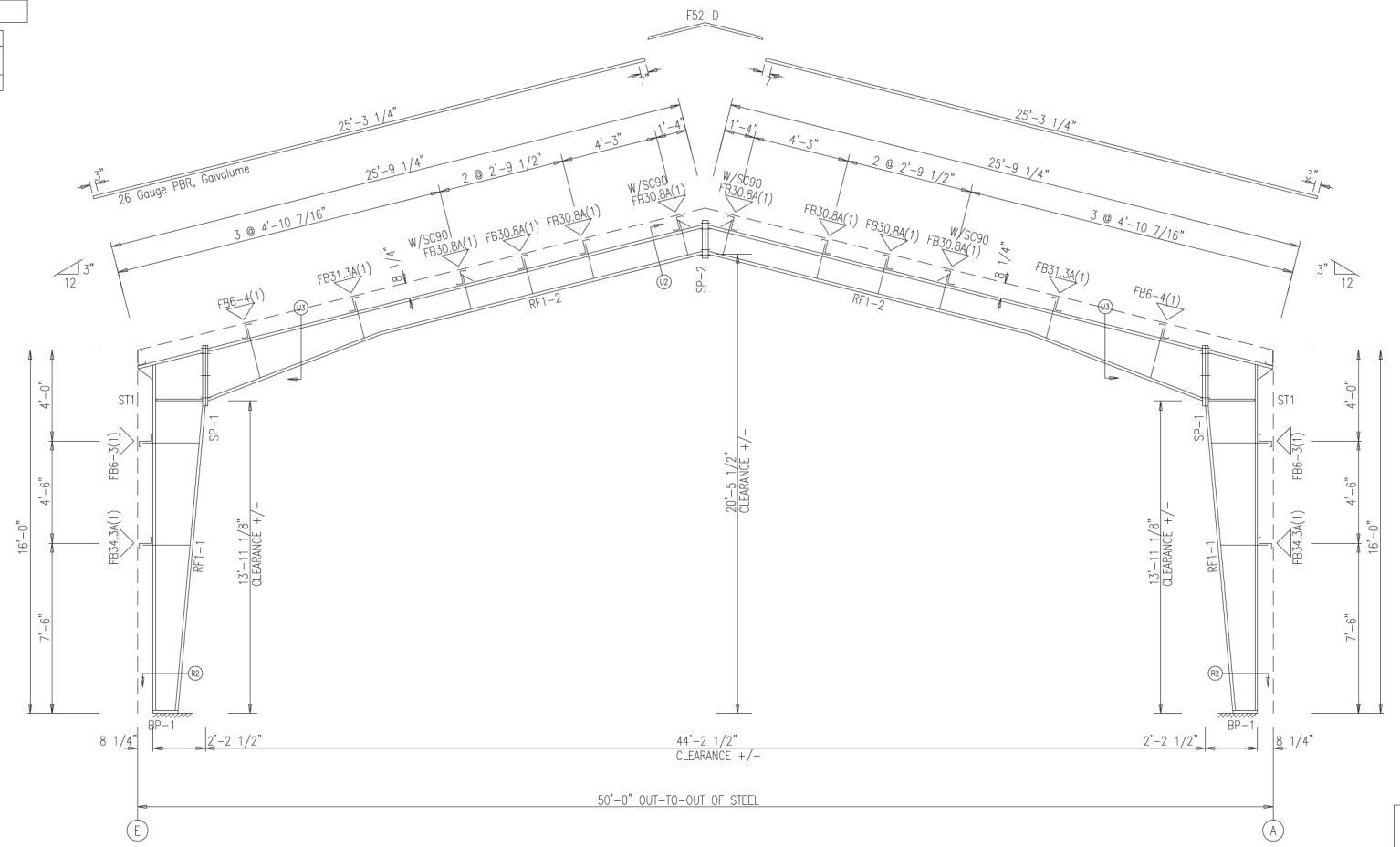
SPLICE PLATE & BOLT TABLE									
Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	2	A325	3/4"	2"	6"	1/2"	2'-7 5/8"
SP-2	4	4	0	A325	3/4"	1 3/4"	6"	3/8"	1'-7 1/8"

STIFFENER TABLE				
Mark	Stiff Mark	Plate Size Width	Thick	Length
RF1-1	ST1	2 1/2	1/4"	26"

BASE PLATE TABLE			
Col Mark	Plate Size Width	Thick	Length
BP-1	6"	3/8"	10 1/2"

FLANGE BRACES: FBxx (1 or 2)
xx=length(in)
(1) One Side; (2) Two Sides
A – L2X2X14G
FB6 – L2X2X1/8

MEMBER TABLE					
Mark	Web Depth Start/End	Web Plate Thick	Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF1-1	10.0/26.0	0.134	191.5	5 x 1/4" x 184.9 5 x 1/4" x 35.5	5 x 1/4" x 164.2
RF1-2	24.0/12.0 12.0/12.0	0.134 0.134	98.3 180.0	5 x 1/4" x 240.0 5 x 1/4" x 32.2	5 x 1/4" x 99.0 5 x 1/4" x 176.9



REFER DETAIL CF02035 FOR SC90 PLATE

RIGID FRAME ELEVATION: FRAME LINE 2 3 4

GENERAL NOTES:

1. BOLT TIGHTENTING–BOLTEDJOINTS WITHASTMA325TYPE 1 BOLTS GREATERTHAN 1/2" DIAMETER ARE SPECIFIED AS PRETENSIONED JOINTS IN ACCORDANCE WITH THE MOST RECENT EDITION OF THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. PRETENSIONING CAN BE ACCOMPLISHED BY USING THE TURN–OF–NUT METHOD OF TIGHTENING, CALIBRATED WRENCH, TWIST–OFF–TYPE TENSION–CONTROL BOLTS OR DIRECT–TENSION–INDICATOR AS ACCEPTABLE TO THE INSPECTING AGENCY AND BUILDING OFFICIAL. INSTALLATION INSPECTION REQUIREMENTS FOR PRETENSIONED JOINTS (SPECIFICATION FOR STRUCTURAL JOINTS SECTION 9.2) USING TURN–OF–NUT METHOD IS SUGGESTED. THE CONNECTIONS ON THIS PROJECT ARE NOT SLIP CRITICAL.
2. ALL FIELD CONNECTIONS OF SECONDARY FRAMING SHALL BE BOLTED WITH A325 BOLTS.
3. INSTALL ALL FLANGE BRACES ON COLUMN AND RAFTER AS SHOWN.

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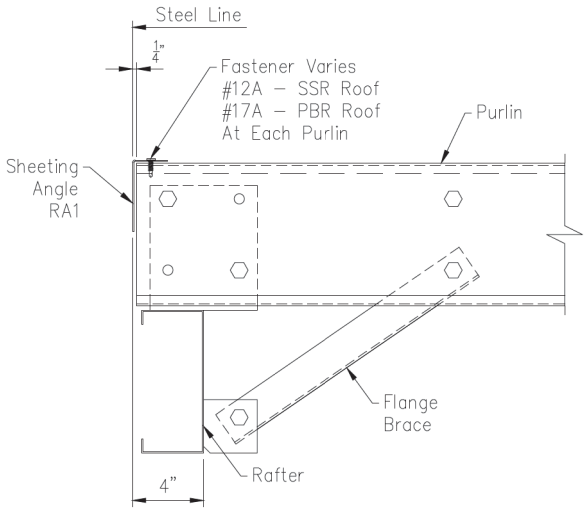
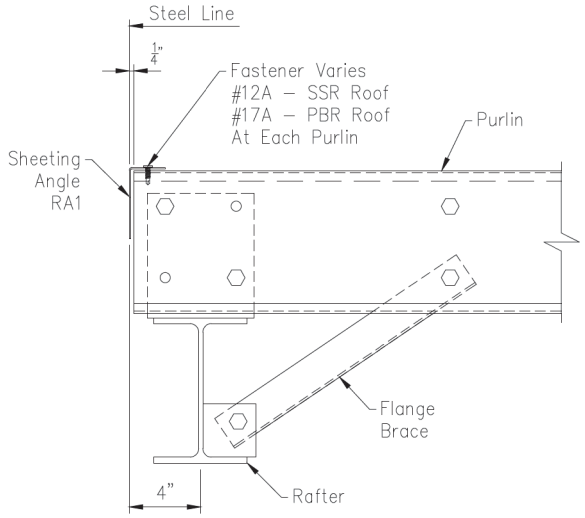


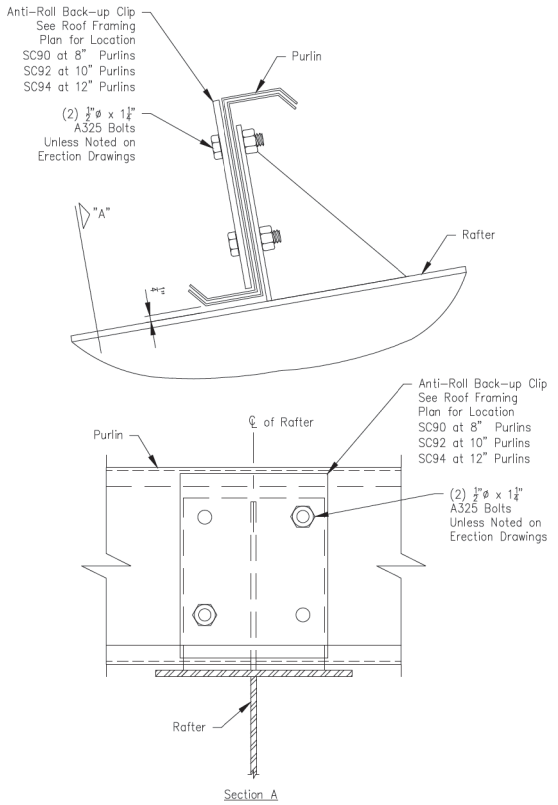
A5	Purlin To Bearing Frame Single Cold Form Rafter	Date Jul '21
Page MB–A5		Rev 02



A7	Purlin To Bearing Frame Hot Rolled Rafter	Date Jul '21
Page MB–A7		Rev 02

Purlin with Anti-Roll Back-up Clip at Rafter

Page CF02035	
Date Dec '14	Rev 04



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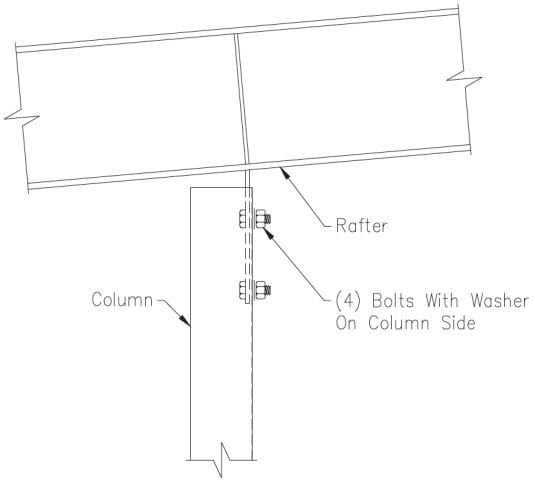
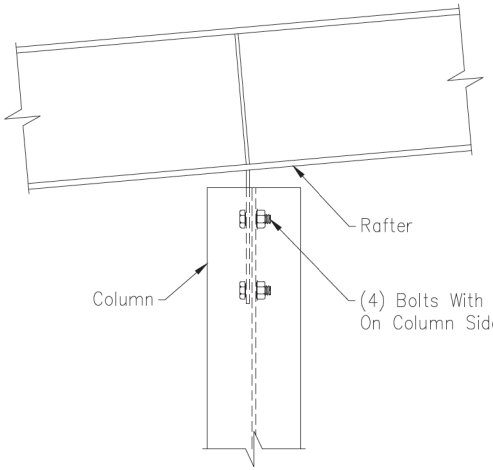
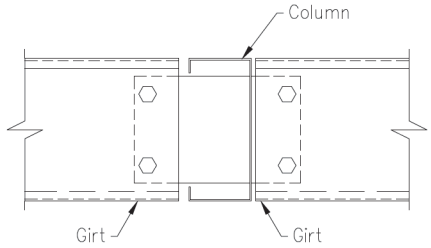


B4	Cold Form Endwall Column To Rafter	Date Aug '20
Page MB-B4		Rev 01



B6	Hot Rolled Endwall Column To Rafter	Date Aug '20
Page MB-B6		Rev 01



C4	Girt To Cold Form Column	Date Jun '17
Page MB-C4		Rev 00

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OWNER: NATALIE DAVIDSON
LOCATION: SPARKS,NV 89441-8549 US

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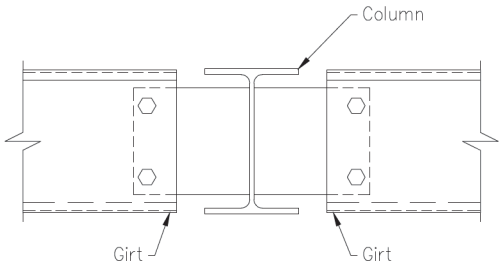
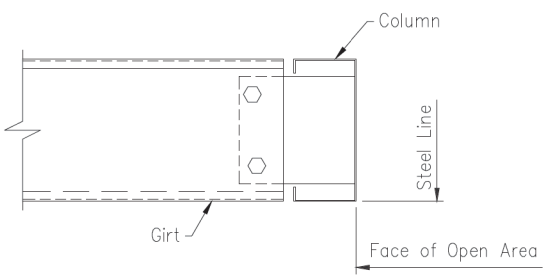
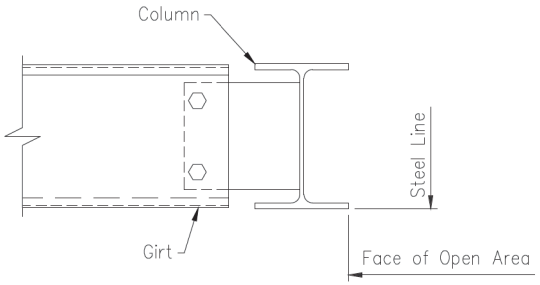


C6	Girt To Hot Rolled Endwall Column	Date Jun '17
Page MB-C6		Rev 00



C13	Girt To Cold Form Endwall Column – Partially Open	Date Jun '17
Page MB-C13		Rev 00



C15	Girt To Hot Rolled Endwall Column – Partially Open	Date Jun '17
Page MB-C15		Rev 00

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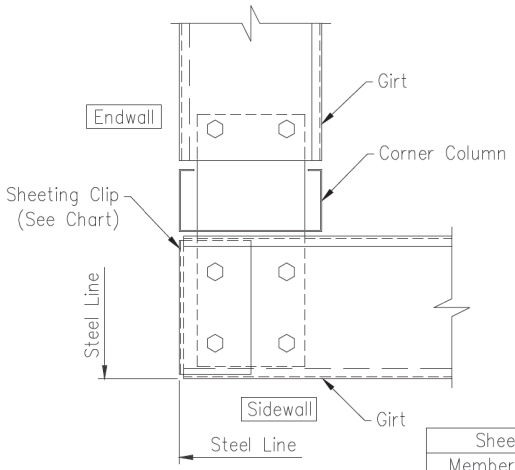
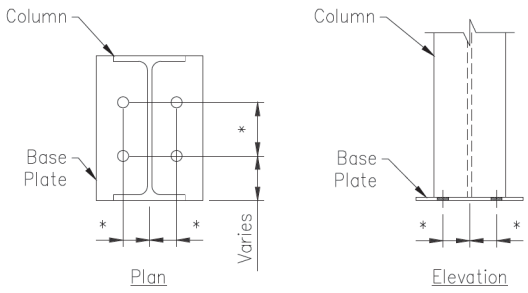
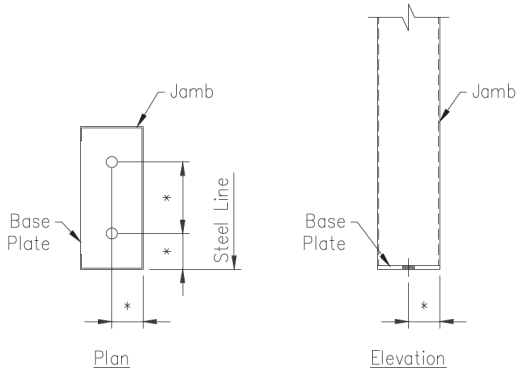


Sheeting Clip	
Member	Piece Mark
8"	SC5
10"	SC54
12"	SC55



* – Refer To Anchor Rod Setting Plan For Dimension



* – Refer To Anchor Rod Setting Plan For Dimension

D4	Girt To Cold Form Corner Column	Date Oct '19	E3	Endwall Column Base Plate	Date Dec '18	E5	Door Jamb Base Plate	Date Dec '18
Page MB-D4		Rev 01	Page MB-E3		Rev 01	Page MB-E5		Rev 01

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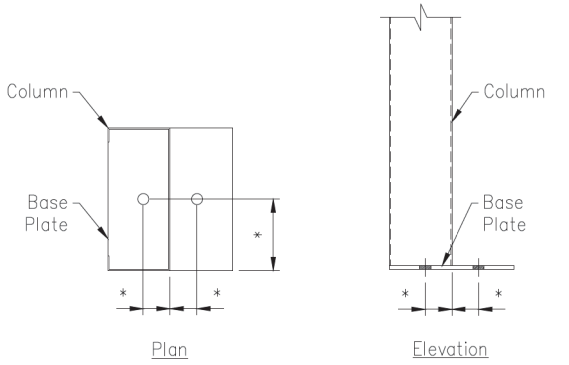
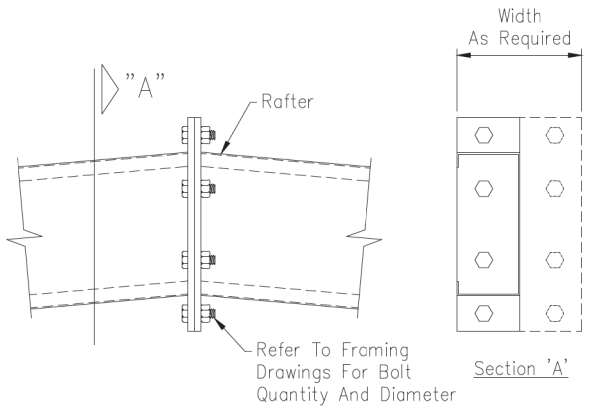
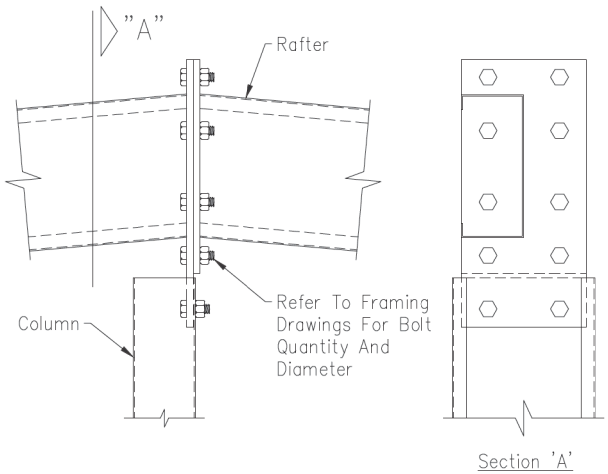


* – Refer To Anchor Rod Setting Plan For Dimension

E8	Cold Form Endwall Column Base Plate	Date Dec '18
Page MB–E8		Rev 01



F10	Endwall Bearing Frame – Cold Form Rafter Splice At Ridge	Date Jun '17
Page MB–F10		Rev 00



F20B	Endwall Bearing Frame – Cold Form Rafter Splice At Ridge	Date Jun '17
Page MB–F20B		Rev 00

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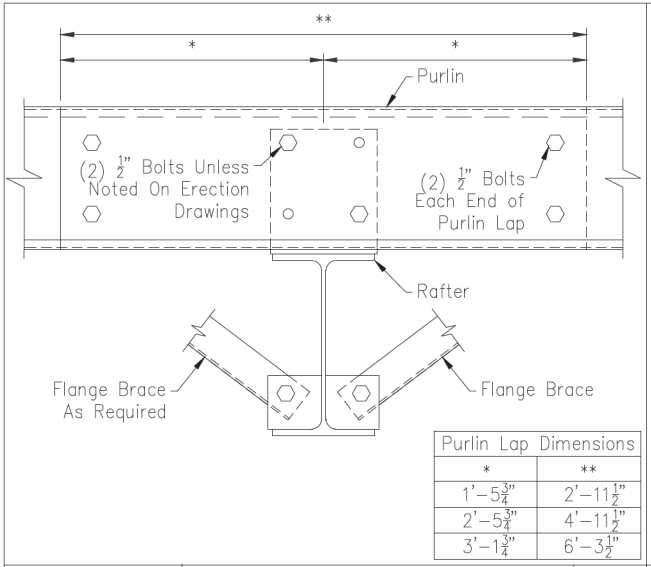
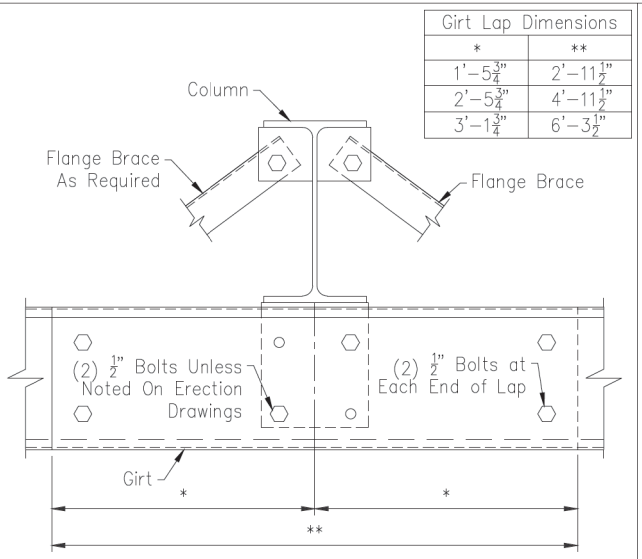


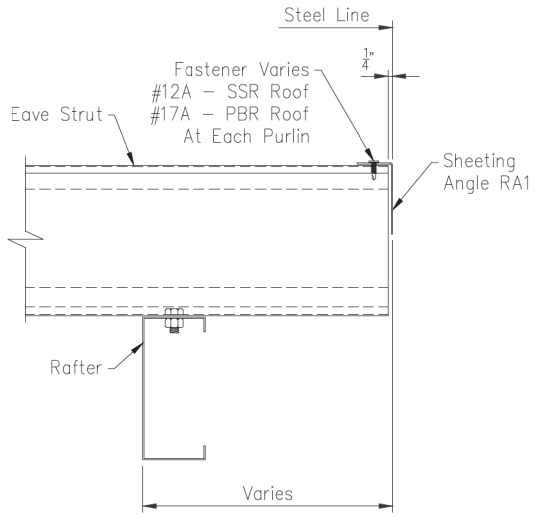
Purlin Lap Dimensions	
*	**
1'-5 3/4"	2'-11 1/2"
2'-5 3/4"	4'-11 1/2"
3'-1 3/4"	6'-3 1/2"

G2	Purlin To Rigid Frame	Date Sep '19
Page MB-G2		Rev 01



Girt Lap Dimensions	
*	**
1'-5 3/4"	2'-11 1/2"
2'-5 3/4"	4'-11 1/2"
3'-1 3/4"	6'-3 1/2"

H2	Girt To Rigid Frame	Date Sep '19
Page MB-H2		Rev 01



I6	Low Side Eave Strut To Bearing Frame - Cold Form	Date Jun '17
Page MB-I6		Rev 00

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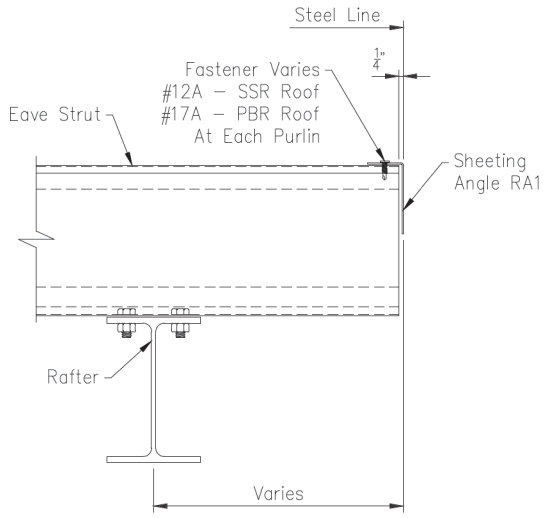
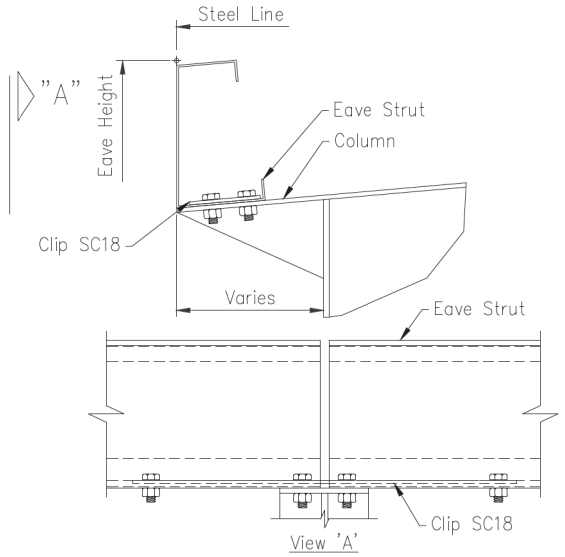
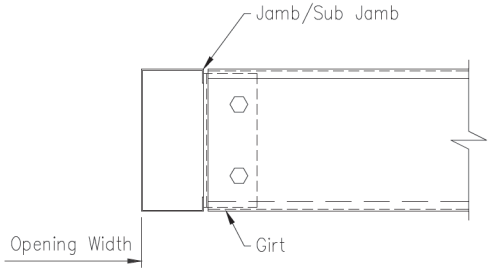


I8	Low Side Eave Strut To Bearing Frame – Hot Rolled	Date Jun '17
Page MB–I8		Rev 00



J4	Eave Strut To By–Pass Rigid Frame At Interior	Date Jun '17
Page MB–J4		Rev 00



K3	Girt To Single Cold Form Jamb/Sub Jamb	Date Dec '17
Page MB–K3		Rev 00

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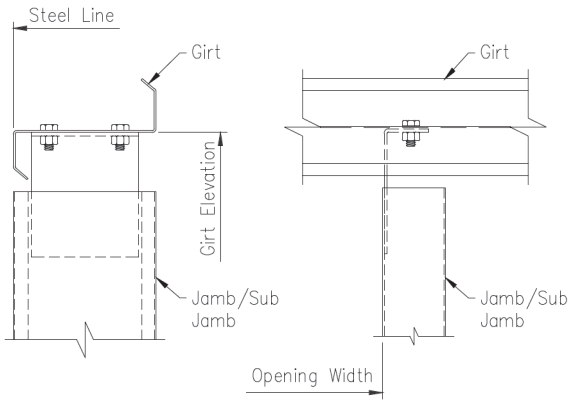

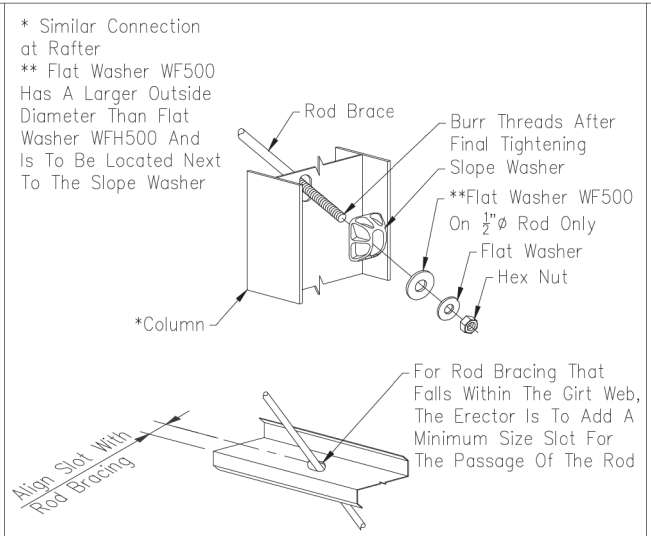

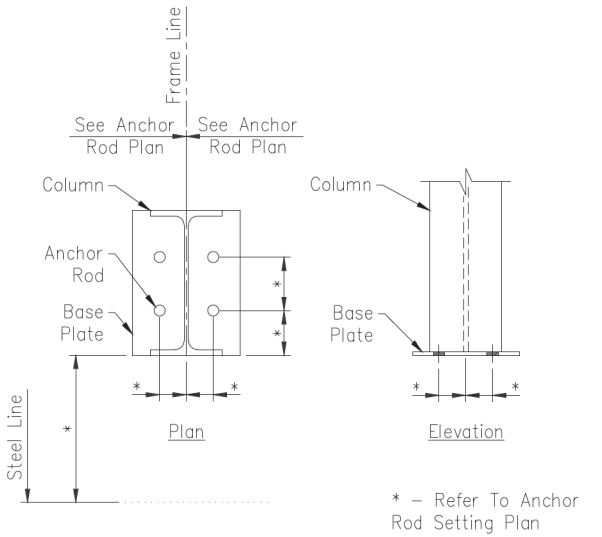

L8	Single Cold Form Jamb/ Sub Jamb To Girt	Date Jun '17	Q3	Rod Brace Attachment At Web	Date Mar '18	R2	Anchor Rods At Frame Column	Date Dec '17
Page MB-L8		Rev 00	Page MB-Q3		Rev 01	Page MB-R2		Rev 00

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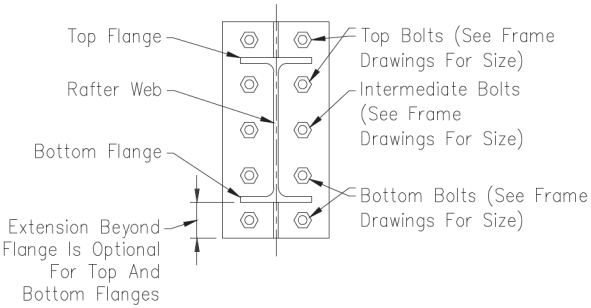
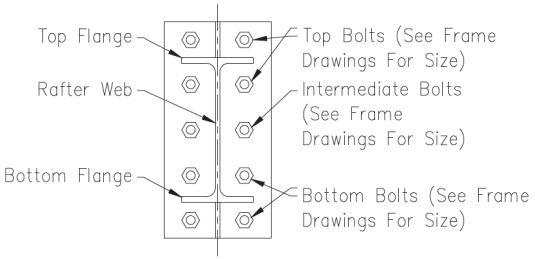
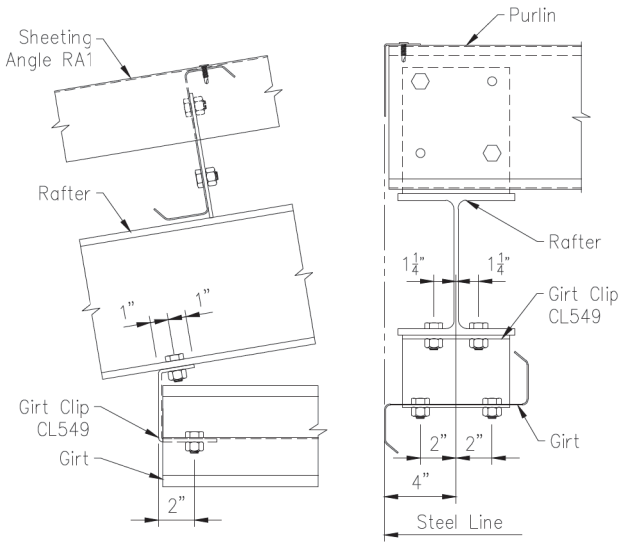


U2	Bolts At Rigid Frame Ridge Rafter Connection	Date Jun '17
Page MB-U2		Rev 00



U3	Bolts At Rigid Frame Rafter To Column Connection	Date Jun '17
Page MB-U3		Rev 00



W10	Girt To Hot Rolled Endwall Rafter	Date May '19
Page MB-W10		Rev 00

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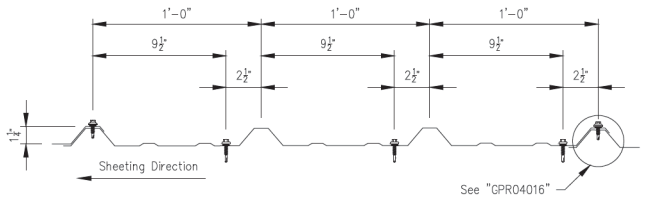
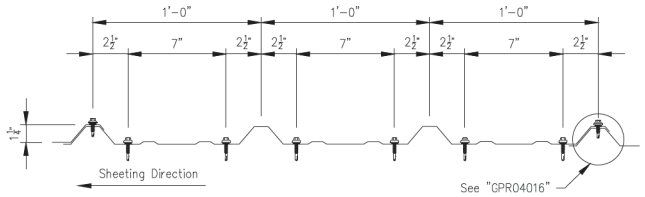
PBR Roof Panel
Fastener And Tape Sealant Location

Page GPR00011 | Date Apr '19 | Rev 01



All Roof Members Except As Noted Below

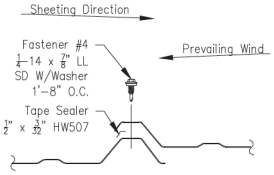


At Eave Strut, Panel End Lap And Peak Purlin

Note:
Screw patterns shown satisfy U.L. 90 requirements for roof.

PBR Roof Panel
Side Lap And End Lap Details

Page GPR04016 | Date Apr '19 | Rev 04



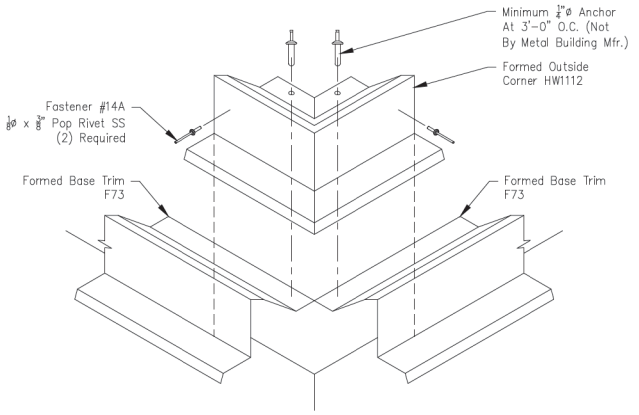
Section Thru Panel Side Lap

Panel Endlap Detail

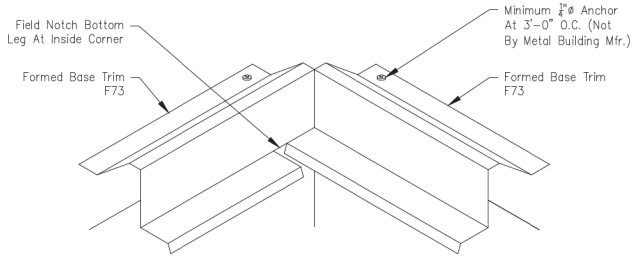
Panel Sidelap Detail
At Low Eave

Formed Base Trim Details

Page PW02010 | Date Feb '18 | Rev 01



Outside Corner Detail



Inside Corner Detail

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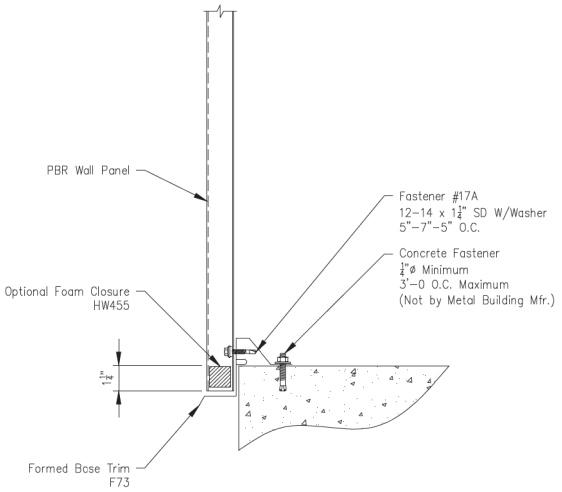
PBR Wall Panel
F73 Formed Base Trim Without Panel Recess

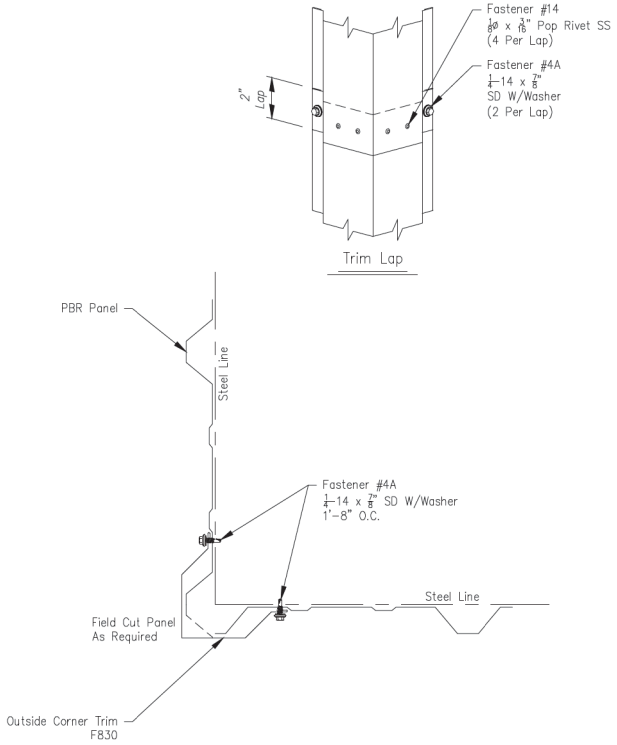
Page PW02107 | Date Jul '17 | Rev 01



Wall panel must be held off of base trim a minimum of 1/4" to prevent bottom of wall panel from rusting.

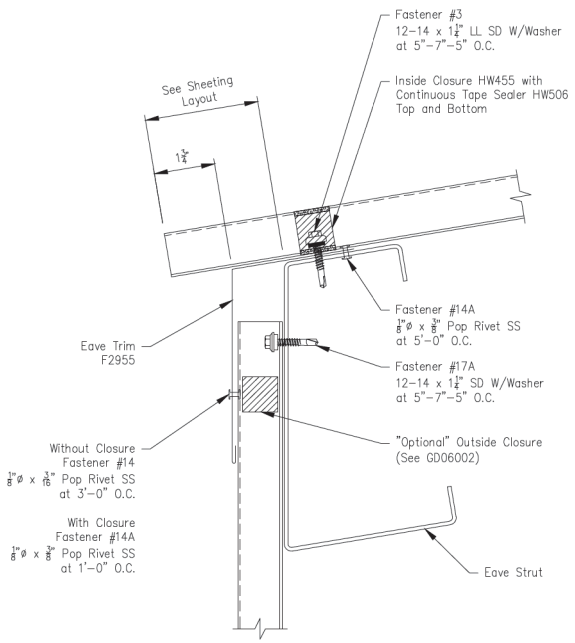
PBR Wall Panel
Outside Corner - On Module

Page PW03001 | Date Apr '19 | Rev 08



PBR Panel
Flat Eave Trim - Sheeted Wall

Page GPR03004 | Date Feb '17 | Rev 05



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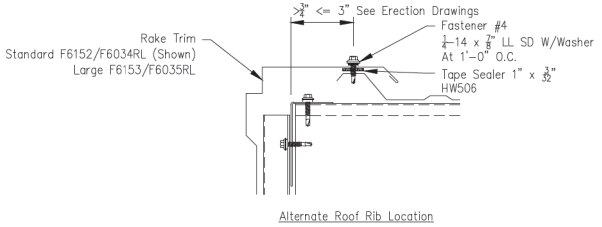
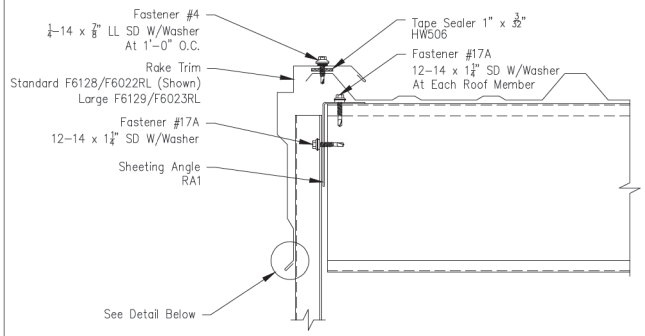
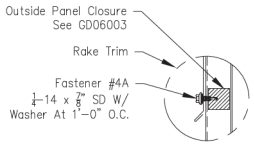
PBR Roof Panel - Northern Standard And Northern Large Edgecraft Rake Trim - Sheeted Wall

Page TPR05006 | Date Jul '20 | Rev 00

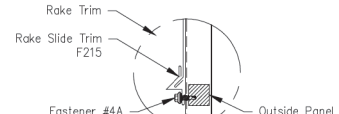


Alternate Roof Rib Location





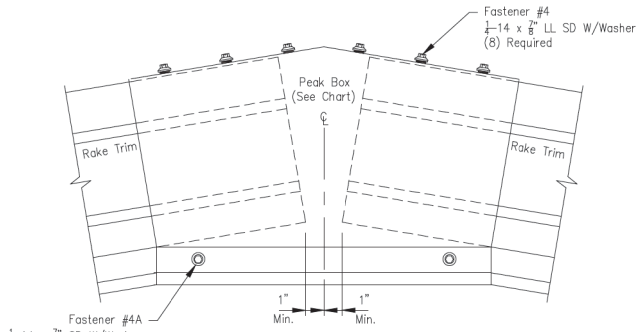
Standard
For Roof Runs 100'-0" Or Less

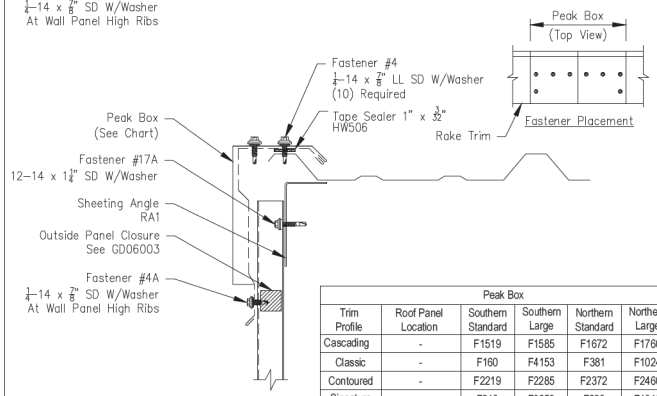


Alternate
For Roof Runs Greater Than 100'-0"

PBR Roof Panel Peak Box At Fixed Ridge

Page GPR05001 | Date Jun '21 | Rev 08



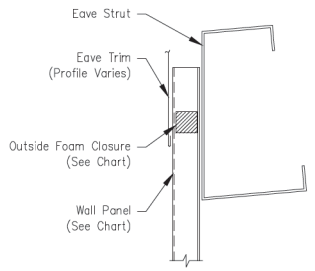


Note:
Flashing profile vcries dependent on the building order. Attachment as illustrated is applicable for all profiles.

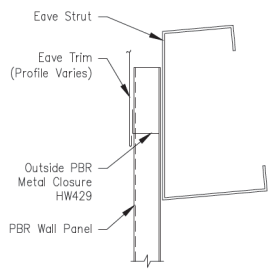
Peak Box					
Trim Profile	Roof Panel Location	Southern Standard	Southern Large	Northern Standard	Northern Large
Cascading	-	F1519	F1585	F1672	F1760
Classic	-	F160	F4153	F381	F1024
Contoured	-	F2219	F2285	F2372	F2460
Signature	-	F916	F3853	F236	F1018
Edgecraft	SL <= 3/4"	F6028	F6029	F6030	F6031
Edgecraft	>3/4" <= 3"	F6036	F6037	F6038	F6039
Sculptured	SL <= 3/4"	F7052	F7055	F7058	F7061
Sculptured	>3/4" <= 3"	F7064	F7067	F7070	F7073

Single Skin Wall Panel Outside Closure Requirements at Eave

Page GD06002 | Date Feb '19 | Rev 02



Detail at Foam Closure
(Low Eave Shown High Eave Similar)



Detail at Optional Metal Closure
For PBR Panel Only
(Low Eave Shown High Eave Similar)

Note:
Foam Closures Are Required When Job Requires Air Infiltration Or Sealed Wall Requirements, See GD16002.

Wall Panel	Foam Closure
PBR	HW456
AVP	HW465
PBU	HW460
VistaShadow NuWall	HW465 HW424
PBC	HW462
PBD	HW463
ShadowRib	HW412
Designer Series (Fluted Only)	HW4037
RBR (Reverse Rolled PBR)	HW455
RBU (Reverse Rolled PBU)	HW459
7.2	HW461

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PROJECT: NATALIE DAVIDSON
CUSTOMER: THE STEEL BUILDER
OWNER: NATALIE DAVIDSON
LOCATION: SPARKS,NV 89441-8549 US

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Feb 07, 2023
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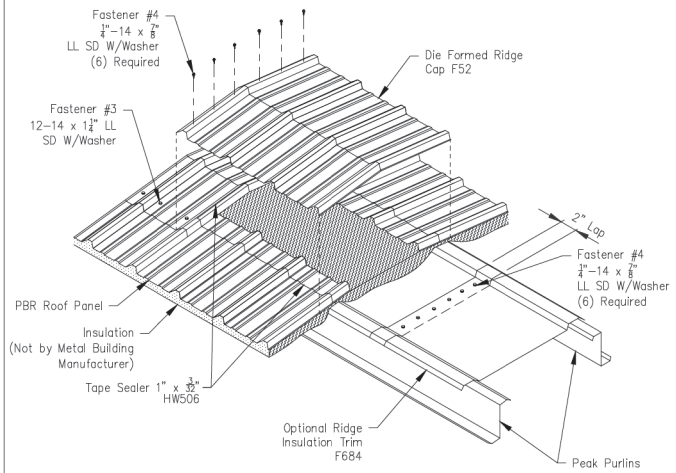
PBR Roof Panel
Fixed Ridge Detail

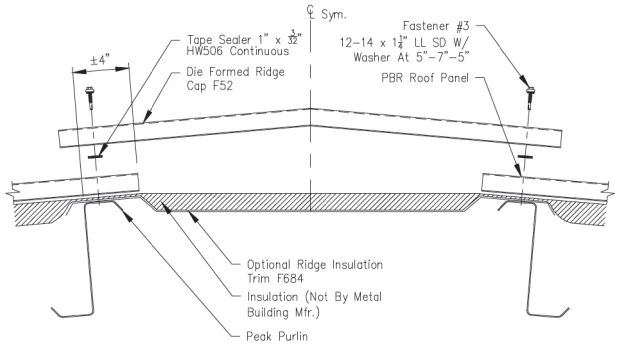
Page GPR06003 | Date Apr '19 | Rev 04



Note:
Install optional ridge insulation trim F684 as insulation and roof sheeting is being applied. Temporary attachment, if required, is not by metal building manufacturer.

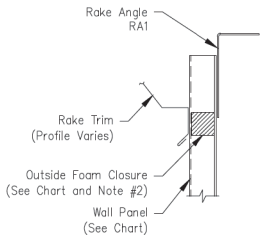
PBR Roof Panel
Section Thru Fixed Ridge

Page GPR06004 | Date Apr '19 | Rev 04

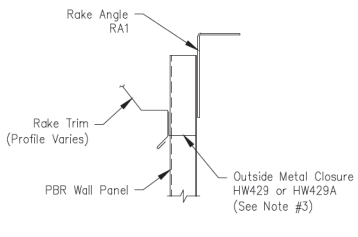


Single Skin Wall Panel Outside Closure Requirements at Rake

Page GD06003 | Date Feb '19 | Rev 02

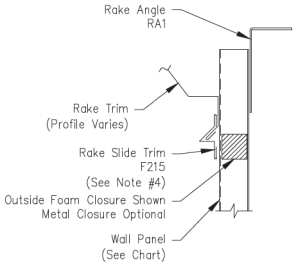


Detail at Foam Closure



Detail at Optional Metal Closure
For PBR Panel Only

Notes:
1. Outside Panel Closures are Required at all Sheeted Endwalls.
2. PBR Wall Panel Outside Foam Closures HW456 Required for Roof Slope 4:12 or Less and HW422 for Roof Slope Greater Than 4:12. Field Form/Notch HW422 to Panel Profile.
3. PBR Wall Panel Outside Metal Closure HW429 for Roof Slope 0 :12 Thru 1 1/2 :12 and HW429A for Roof Slope Greater Than 3 1/2 :12 Thru 4 1/2 :12.
4. Rake Slide Trim Required for All Standing Seam Roofs and at Screw Down Roof Runs Greater Than 100'–0"
5. Foam Closures are Required when Job Requires Air Infiltration or Sealed Wall Requirements, See GD16002.



Detail at Rake Slide Trim

Wall Panel	Foam Closure
PBR	HW456/HW422
AVP	HW465
PBU	HW460
VistaShadow	HW465
NuWall	HW424
PBC	HW462
PBD	HW463
ShadowRib	HW412
Designer Series (Fluted Only)	HW4037
RBR (Reverse Rolled PBR)	HW455
RBU (Reverse Rolled PBU)	HW459
7.2	HW461

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OWNER: NATALIE DAVIDSON
LOCATION: SPARKS,NV 89441–8549 US

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Screw Application

Page TH06006X | Date May '19 | Rev 01

Standard Grade

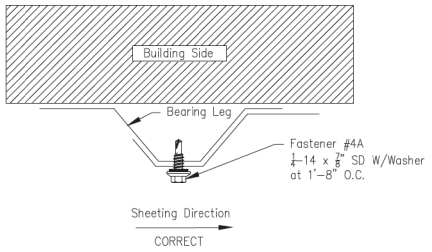
Description	Fastener Number	Application
1/4"–14 x 7/8" Type 2	4A	Stitch & Trim Screw
12–14 x 1 1/4" Type 2	17A	Member Screw (Up To 4" Insulation)
12–14 x 1 1/2" Type 2	17B	Member Screw (Up To 6" Insulation)
12–14 x 2" Type 2	28	Member Screw

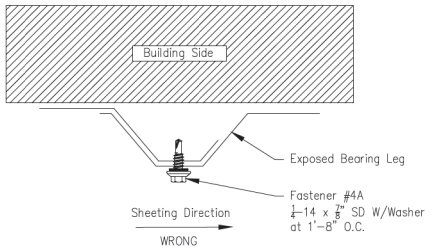
Long Life

Description	Fastener Number	Application
1/4"–14 x 7/8" Type 1	4	Stitch & Trim Screw
12–14 x 1 1/4" Type 2	3	Member Screw (Up To 4" Insulation)
12–14 x 1 1/2" Type 2	3A	Member Screw (Up To 6" Insulation)
12–14 x 2" Type 2	58	Member Screw

PBR Wall Panel
Panel Side Lap

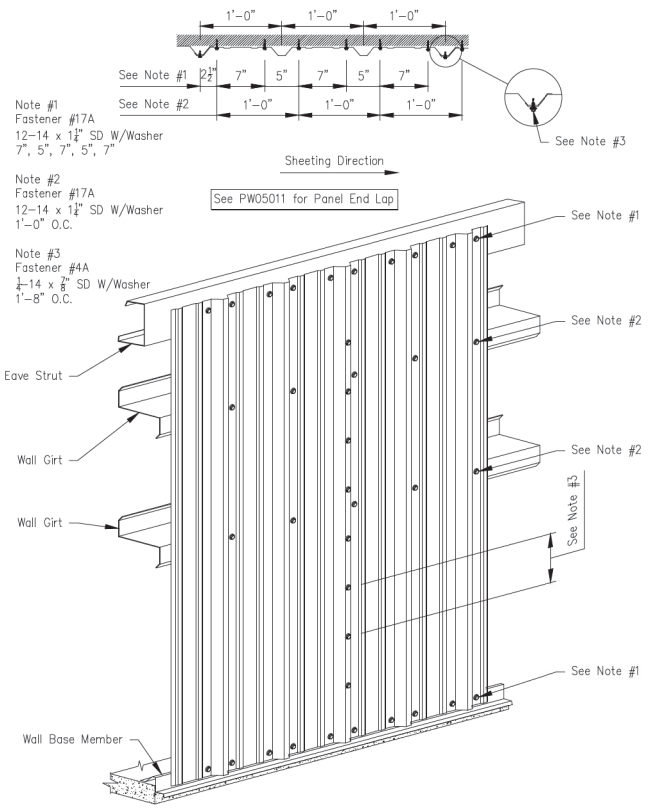
Page PW05002 | Date Mar '19 | Rev 03





PBR Wall Panel
Fastener Location

Page PW05003 | Date Aug '15 | Rev 04



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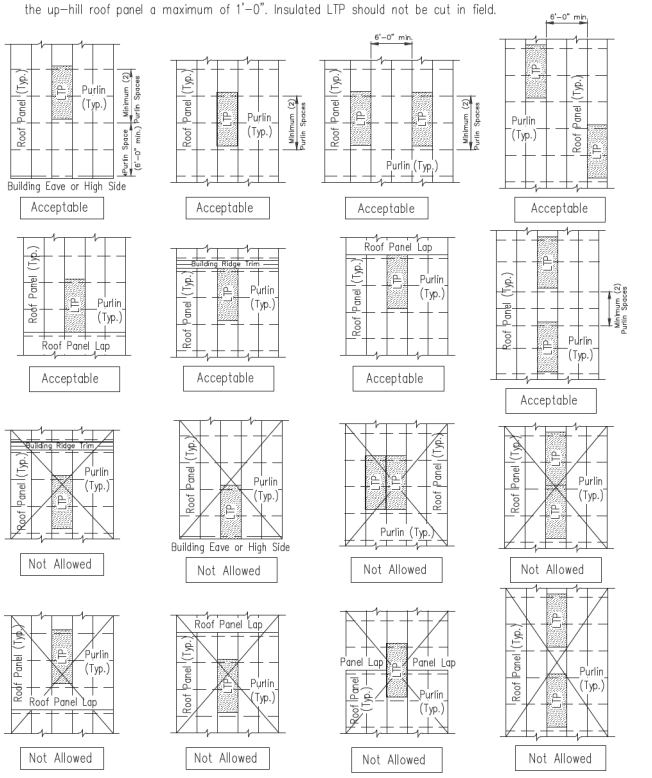
Light Transmitting Panel (LTP)
PBR Roof Panel
Standard Placement Guidelines

Page GPR25102 | Date Apr '17 | Rev 00

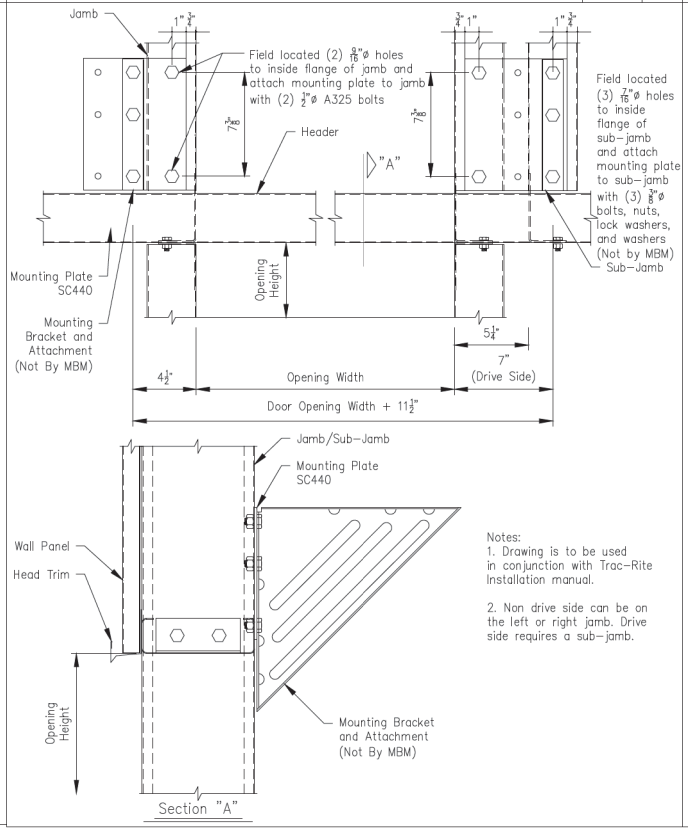
*Note: Roof Panel and Light Transmitting Panel to span a minimum of (2) purlin spaces.

A minimum of 4" is required for panel endlaps. The non-insulated LTP may be field cut or endlap under the up-hill roof panel a maximum of 1'-0". Insulated LTP should not be cut in field.



Trac-Rite Model 988 / 988WL & Model 977 / 977WL Roll-Up Door Mounting Plate Installation

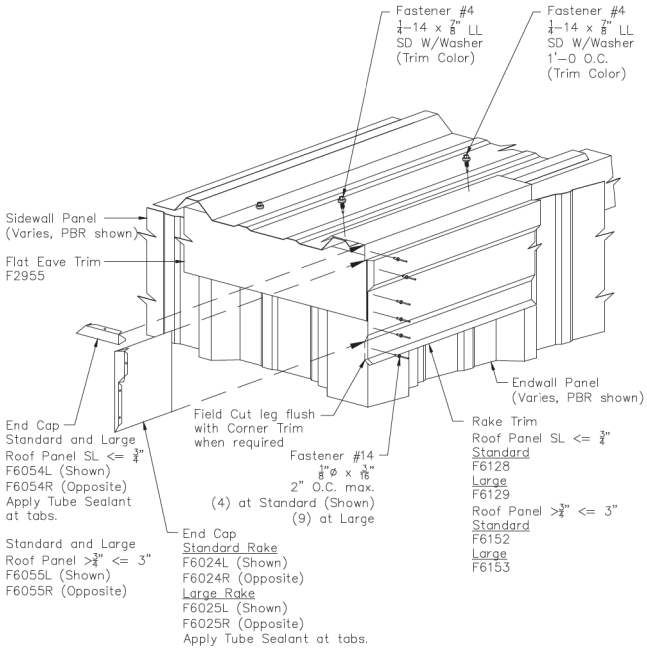
Page AC00835X | Date | Rev



Notes:
1. Drawing is to be used in conjunction with Trac-Rite Installation manual.
2. Non drive side can be on the left or right jamb. Drive side requires a sub-jamb.

PBR Roof Panel - Northern Standard and Northern Large Edgecraft Low Eave Rake Corner with Flat Eave Trim -3/4" thru 1 3/4" Wall Panel

Page TPR04009 | Date Nov '20 | Rev 02



See Construction Details for required attachment of Eave Trim and Rake Trim to Wall. Wall Panel Closures not shown for clarity.

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LOCATION: SPARKS,NV 89441-8549 US

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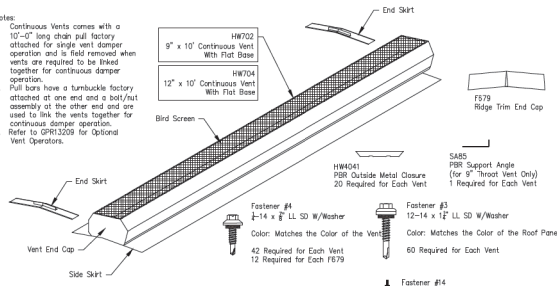
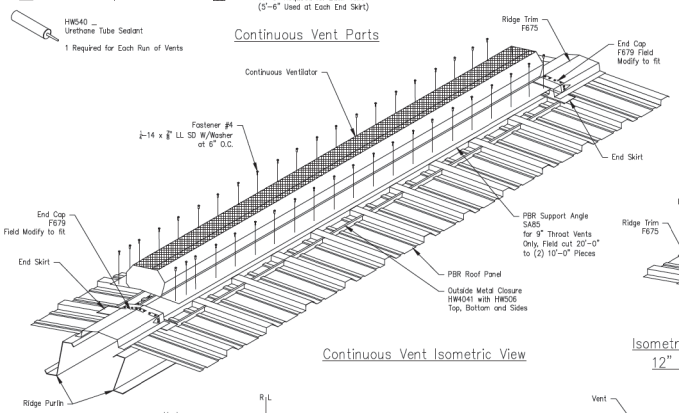
No. 17245

Notes:
1. Continuous Vents comes with a 10'-0" long chain pull factory attached for single vent damper operation and is field removed when vents are required to be linked together for continuous damper operation.
2. Pull bars have a turnbuckle factory attached at one end and a bolt/nut assembly at the other end and are used to link the vents together for continuous damper operation.
3. Refer to GPR13209 for Optional Vent Operators.



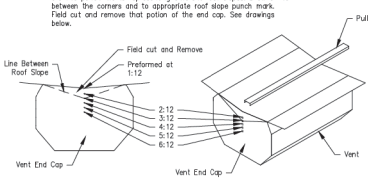
Continuous Vent Parts



Continuous Vent Isometric View

Vent Preparation Before Installation:
The vent end caps are preformed for a 1:12 roof slope and require field modification to the end caps for roof slopes greater than 1:12 prior to installation. Field modification of the vent end cap is not required for roof slopes 1:12 and less.

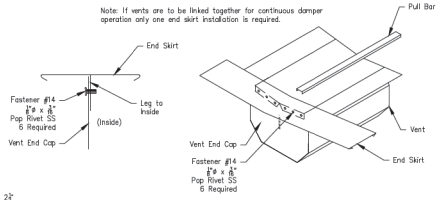
Turn the ventilator over and place the vent on it's top. There are five punch marks representing different roof slopes. Draw a line between the corners and to appropriate roof slope punch mark. Field cut and remove that potion of the end cap. See drawings below.

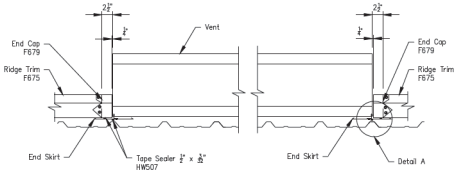


End Skirt Installation:
Position end skirt onto the vent end cap. Be sure the down-turned angle of the end skirt is positioned on the inside and tight against the vent end cap.

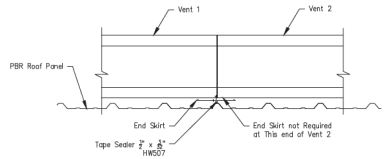
Attach the end skirt to the vent end cap with (6) Fastener #14. See drawing below.

Note: If vents are to be linked together for continuous damper operation only one end skirt installation is required.

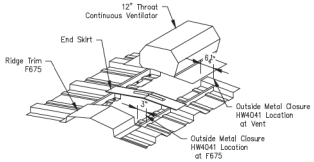




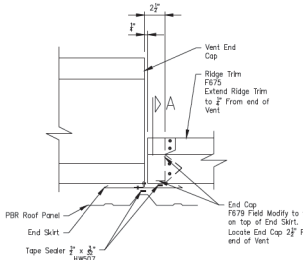
End Skirt Detail for Single Vent



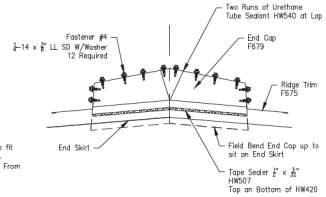
End Skirt Detail for Multiple Vents End to End



Isometric View HW4041 Location at 12" Throat Continuous Vent

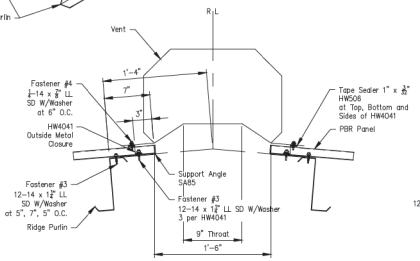


Detail A

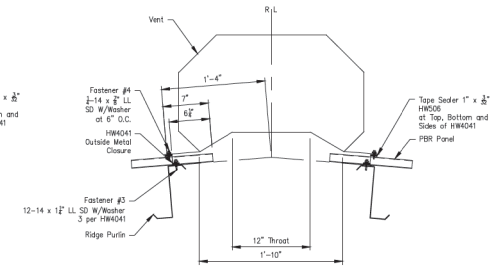


Section A

Refer to GPR06000 for General End Cap Installation Details



Section at Ridge 9" Throat Vent



Section at Ridge 12" Throat Vent

NOTE:
The Metal Building Manufacturer does not recommend the use of Ridge Ventilators on PBR roof systems on gable buildings over 200'-0" in width (100'-0" max. roof plane) or with roof slopes less than 1:12 or greater than 6:12.

Continuous Ridge Ventilator 9" or 12" Throat x 10'-0" Flat Base PBR Roof Panel Greater Than 2 1/2:12 Roof Slope or With Expansion Ridge Trim	Page	GPR13202
	Date	Rev
	Nov '17	01

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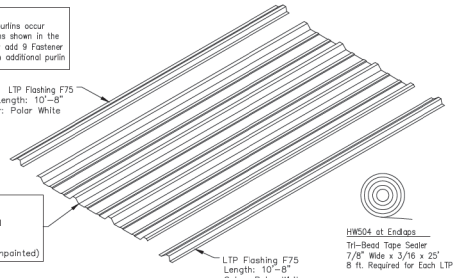
NOTE:
If additional purlins occur between purlins shown in the isometric view add 9 Fastener #43L for each additional purlin



HW504 at Endlaps
Tri-Bead Tape Sealer
7/8" Wide x 3/16 x 25'
8 ft. Required for Each LTP


Fastener #44L
1/4-14 x 3/4" LL SD w/1" Washer
Color: Polar White
26 Required for Each LTP
Sidelap Fastener


Fastener #43L
1/4-14 x 1" LL SD w/1" Washer
Color: Polar White
18 Required for Each LTP


Fastener #3
12-14 x 1½" LL SD w/Washer
Color: Match Roof Panel Color/Finish
9 Required for Each LTP
LTP to Purlin Fastener


HW507 at Sidelaps
Tape Sealer
1/2" Wide x 3/32 x 50'
25 ft. Required for Each LTP

GENERAL NOTES:

1. The light transmitting panels are not designed or intended to bear the weight of any person walking, stepping, standing, or resting on them. THE MANUFACTURER DISCLAIMS ANY WARRANTY OR REPRESENTATION, EXPRESSED OR IMPLIED, that any person can safely walk, step, stand, or rest on or near the light transmitting panels, or that they comply with any OSHA regulation. It is the Users responsibility to ensure that the installation and use of the light transmitting panels comply with State, Federal and OSHA regulations and laws, including, but not limited to, guarding all light transmitting panels with screens, fixed standard railings, or other acceptable safety controls that prevent fall-through.

2. Non-insulated light transmitting panels may need to be cut in the field, if required, follow these steps:
Up to five (5) LTP's may be cut at one time.
A circular saw with a 24 tooth per inch blade, will work best.
Cut slow and allow the blade to work without excess pressure.
Allow the blade to cool between cuts.

3. It is suggested to pre-drill the light transmitting panels before installing fasteners. This will help prevent fractures which may cause possible leaks.

4. Remove drill shavings and metal filings from the surface of the panels at the end of each work day. Rust caused by these items can destroy the panel finish and void warranties.

INSTALLATION NOTES:

1. Install roof panel and insulation according to standard procedures up to the desired location of the LTP shown on the "ROOF SHEETING PLAN". Install the LTP Flashing F75 at both sides of the LTP opening. Apply double face tape (not provided by Metal Bldg. Mfr.) to the sides of the LTP Flashing F75 to secure the insulation see Sections A and B.

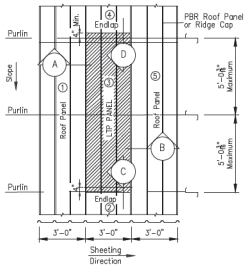
2. Install insulation above and below the LTP opening. Apply double face tape to the purlins to secure the insulation see Sections C and D.

3. Apply Tape Sealer HW507 at the panel sidelap as shown in Section A and the isometric view.

4. Apply Tape Sealer HW504 on top of the lower roof panel endlap (panel ②) and extend Tape Sealer HW507 6" over the top of the roof panel sidelap Tape Sealer HW507. See Section C and the isometric view.

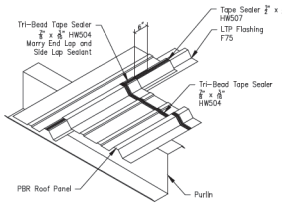
5. Apply Tape Sealer HW507 on top of the LTP sidelap and Tape Sealer HW504 at upper endlap. Extend Tape Sealer HW507 6" over the top of the lower roof panel sidelap Tape Sealer HW507 and the upper roof panel endlap (panel ④). See section D and the isometric view.

6. Attach LTP panel at sidelaps with sidelap fastener at 10" o.c. and endlaps and purlins as shown in "Endlap and Purlin Fastener Layout".



Standard LTP Location

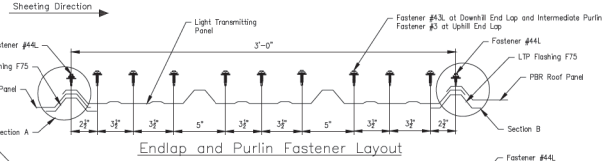
See GPR25102 for Standard Placement Guidelines



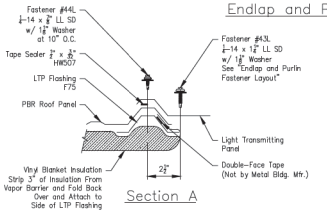
End Lap Sealant Detail

PBR Light Transmitting Panel Assembly Parts

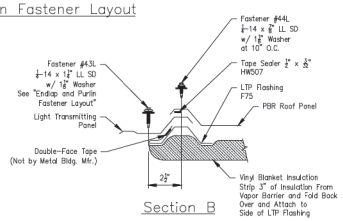
U.L. 90 Rated Roof Construction Number 542



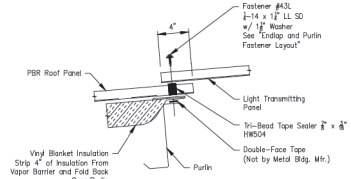
Endlap and Purlin Fastener Layout



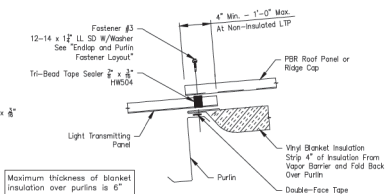
Section A



Section B

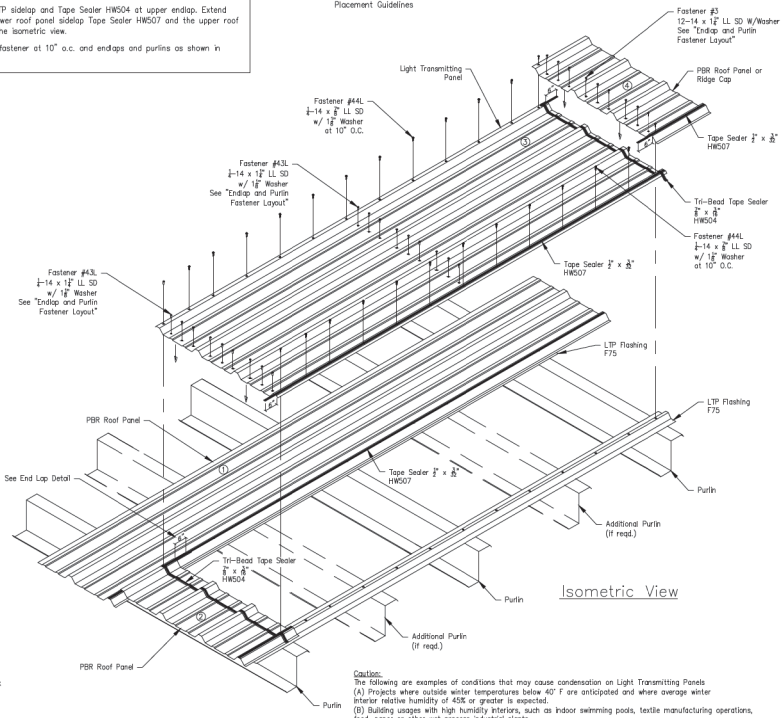


Section C



Maximum thickness of blanket insulation over purlins is 6"

Section D



Isometric View

Caution:
The following are examples of conditions that may cause condensation on Light Transmitting Panels
(A) Projects where outside winter temperatures below 40° F are anticipated and where average winter interior relative humidity of 45% or greater is expected.
(B) Building usages with high humidity interiors, such as indoor swimming pools, textile manufacturing operations, food, paper or other wet process industrial plants.
(C) Construction elements that may release moisture after the roof is installed, such as interior concrete and masonry, plaster finishes, and fuel burning heaters.The Building Manufacturer is not responsible for determining if condensation will be an issue on any particular application.

Non-Insulated PBR Light Transmitting Panel (LTP)
Non UL 90 Application or UL 90 Application

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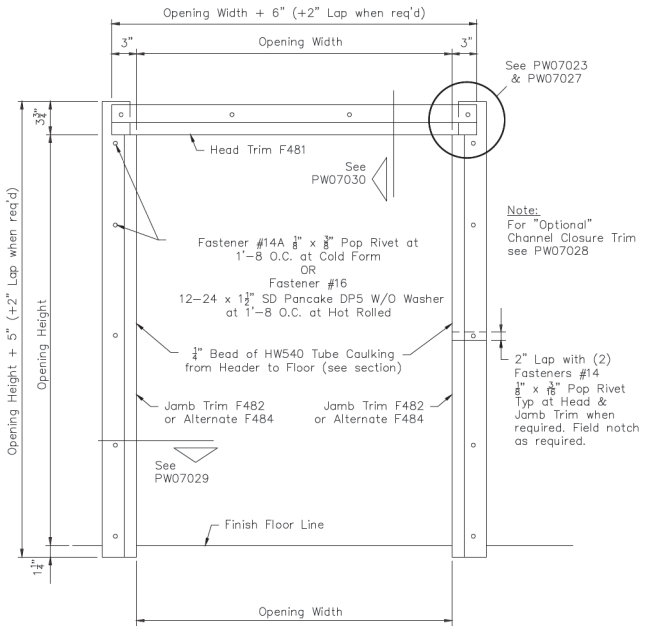
PBR Wall Panel - Three Sided Framed Opening - Trim Installation with Field Notch Panel at Head Trim

Page PW07022 | Date Mar '20 | Rev 05

Note: Trim Installation can be done by Field Notch Panel as shown on PW07022 & PW07023 OR with Field Notch and Bend Tabs at Head Trim as shown on PW07024 & PW07025.



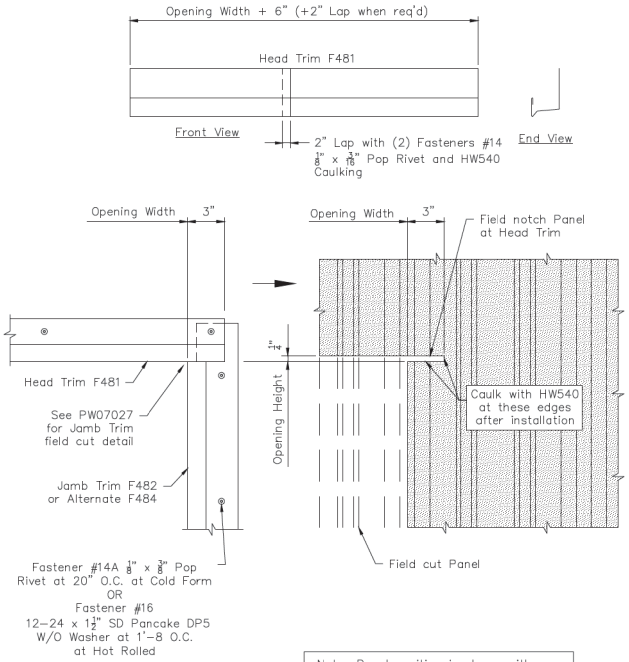
Note: All trim is to be installed BEFORE blanket insulation is applied to walls.

Note: Field measure Opening Width and Height before making field cuts and adjust cut dimensions accordingly.

PBR Wall Panel - Three Sided Framed Opening Field Notch Panel at Head Trim

Page PW07023 | Date Mar '20 | Rev 05

Note: Trim Installation can be done by Field Notch Panel as shown on PW07022 & PW07023 OR with Field Notch and Bend Tabs at Head Trim as shown on PW07024 & PW07025.



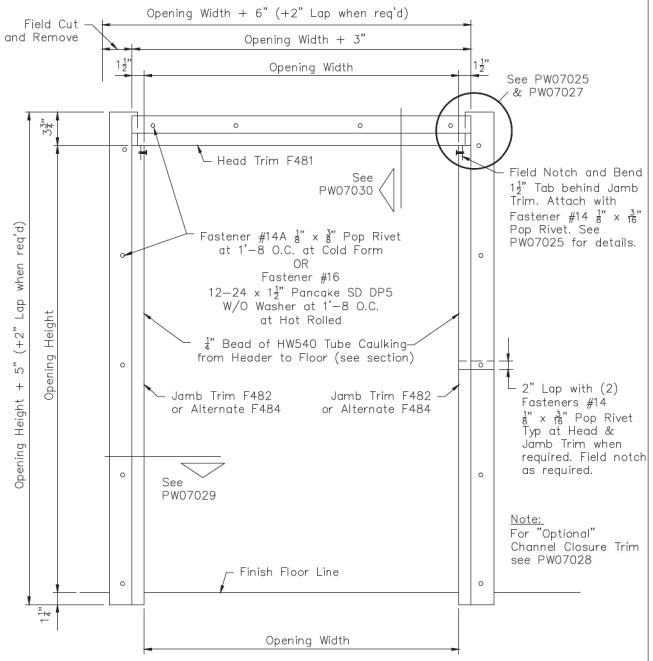
Note: All trim is to be installed BEFORE blanket insulation is applied to walls

Note: Panel position is shown with Panel Rib and Opening on 1'-0 module. Location of Rib may vary depending on the Opening Width and location. Field measure before cutting Panel and Trim.

PBR Wall Panel - Three Sided Framed Opening - Trim Installation with Field Notch and Bend Tabs at Head Trim

Page PW07024 | Date Mar '20 | Rev 05

Note: Trim Installation can be done by Field Notch Panel as shown on PW07022 & PW07023 OR with Field Notch and Bend Tabs at Head Trim as shown on PW07024 & PW07025.



Note: All trim is to be installed BEFORE blanket insulation is applied to walls.

Note: Field measure Opening Width and Height before making field cuts and adjust cut dimensions accordingly.

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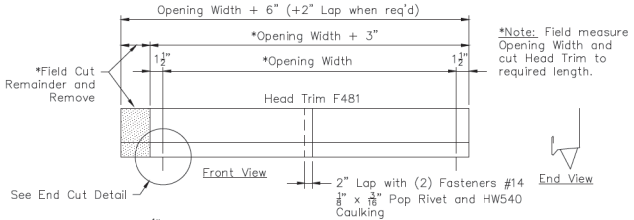
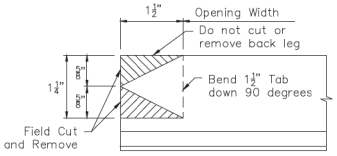
PBR Wall Panel - Three Sided Framed Opening - Field Notch and Bend Tabs at Head Trim

Page PW07025 | Date Mar '20 | Rev 05

Note: Trim Installation can be done by Field Notch Panel as shown on PW07022 & PW07023 OR with Field Notch and Bend Tabs at Head Trim as shown on PW07024 & PW07025.

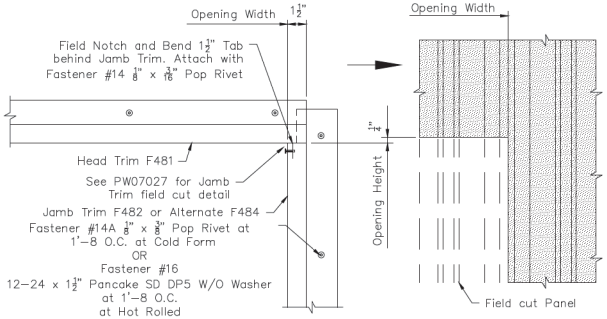




End Cut Detail
(Viewed from top of Head Trim)

Note: All trim is to be installed BEFORE blanket insulation is applied to walls

Note: Panel position is shown with Panel Rib and Opening on 1'-0 module. Location of Rib may vary depending on the Opening Width and location. Field measure before cutting Panel and Trim.



PBR Wall Panel - Three Sided Framed Opening Jamb Trim Field Cut Details

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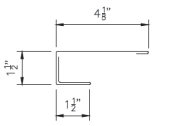
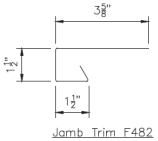
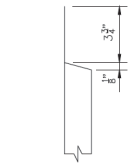
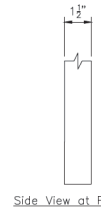
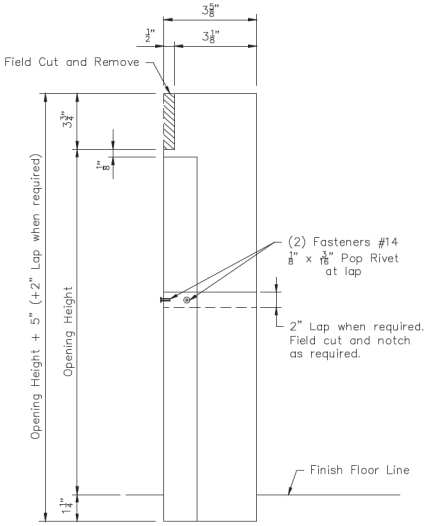








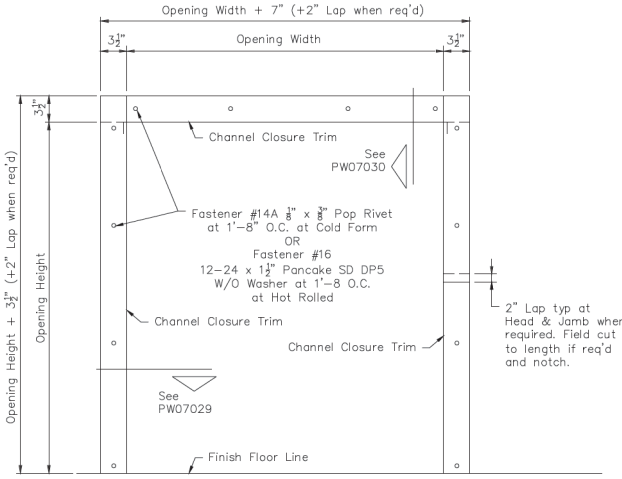


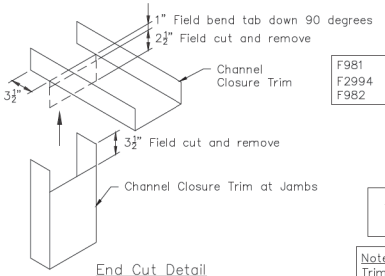
Jamb Trim F482 and Alternate Jamb Trim F484
Front View
Right Jamb Trim as shown
Left Jamb Trim opposite hand

Note: Field measure Opening Height before making field cuts and adjust cut dimensions accordingly.

PBR Wall Panel - Three Sided Framed Opening "Optional" Channel Closure Trim

Page PW07028 | Date May '19 | Rev 04





End Cut Detail

Optional Channel Closure Trim Piece Mark

F981	– 8" Member	F2993	– 10¼" Member
F2994	– 8¼" Member	F169	– 12" Member
F982	– 10" Member	F2995	– 12¼" Member

Note: All trim is to be installed BEFORE blanket insulation is applied to walls.

Note: Field measure Opening Width and Height before making field cuts and adjust cut dimensions accordingly.

Note: The interior leg of the Channel Closure Trim is to be oriented (±1/8") to match the interior leg of the Header or Jamb.

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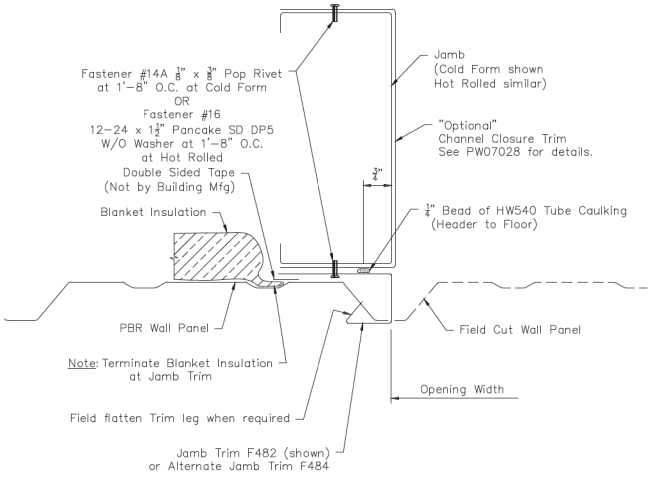
PBR Wall Panel - Three Sided Framed Opening - Jamb Trim Installation

Page PW07029 | Date Mar '20 | Rev 05



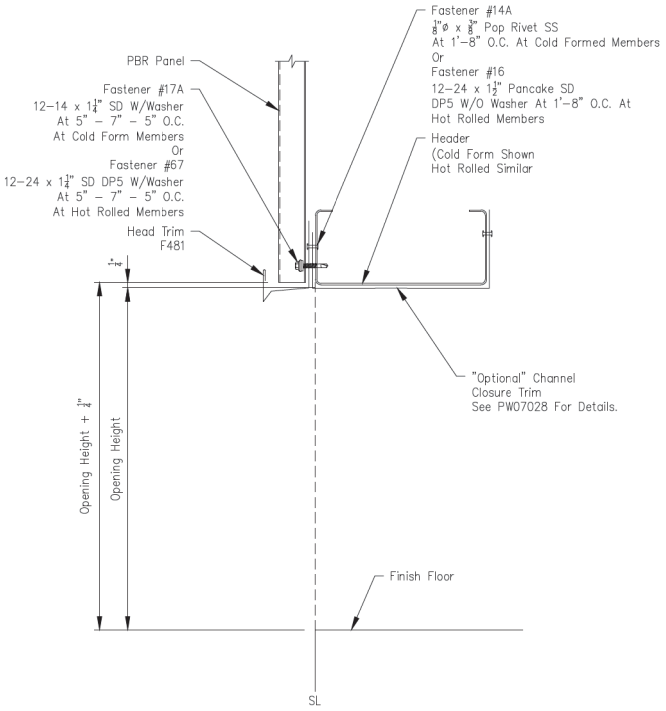
Alternate Jamb Trim F484 Profile

Note: All trim is to be installed BEFORE blanket insulation is applied to walls.

Note: Panel position is shown with Panel Rib and Opening on 1'-0" module. Location of Rib may vary depending on the Opening Width and location. Field measure before cutting Panel and Trim.

PBR Wall Panel - Three Sided Framed Opening Head Trim Installation

Page PW07030 | Date Oct '19 | Rev 03



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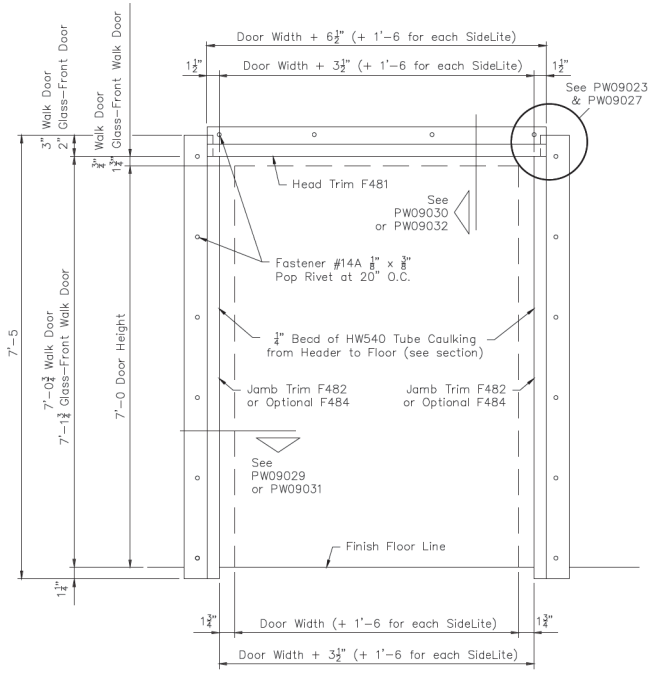
PBR Wall Panel - Walk Door & Glass-Front Walk Door - Trim Installation with Field Notch Panel at Head Trim

Page PW09022 | Date Mar '20 | Rev 03

Note: Trim Installation can be done by Field Notch Panel as shown on PW09022 & PW09023 OR with Field Notch and Bend Tabs at Head Trim as shown on PW09024 & PW09025.



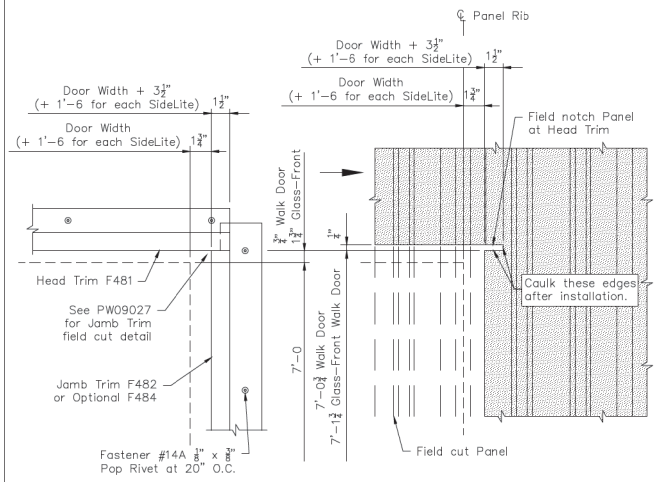
Note: All trim is to be installed BEFORE blanket insulation is applied to walls.

Note: Field measure Door Width and Height before making field cuts and adjust cut dimensions accordingly.

PBR Wall Panel - Walk Door And Glass-Front Walk Door Field Notch Panel at Head Trim

Page PW09023 | Date Mar '20 | Rev 02

Note: Trim Installation can be done by Field Notch Panel as shown on PW09022 & PW09023 OR with Field Notch and Bend Tabs at Head Trim as shown on PW09024 & PW09025.



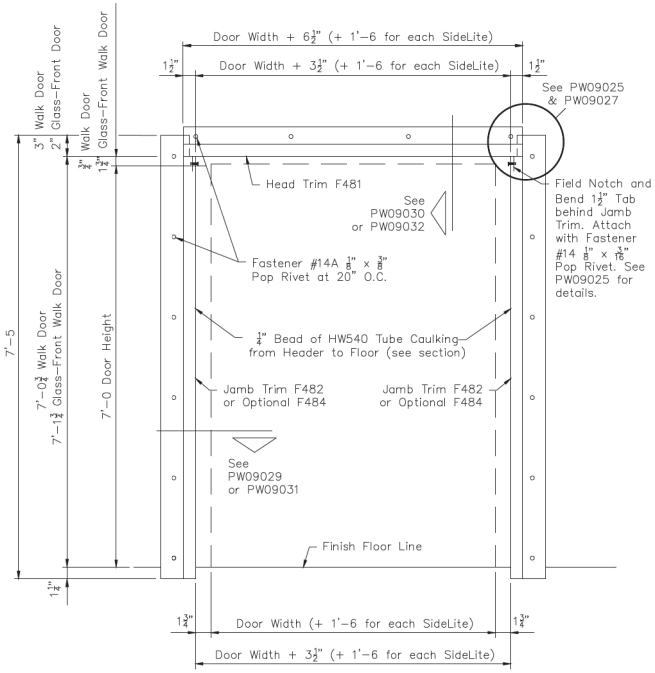
Note: All trim is to be installed BEFORE blanket insulation is applied to walls.

Note: Panel position is shown with Panel Rib and Door on 1'-0 module. Location of Rib may vary depending on the Door Width and location. Field measure before cutting Panel and Trim.

PBR Wall Panel - Walk Door & Glass-Front Walk Door - Trim Installation with Field Notch and Bend Tabs at Head Trim

Page PW09024 | Date Mar '20 | Rev 03

Note: Trim Installation can be done by Field Notch Panel as shown on PW09022 & PW09023 OR with Field Notch and Bend Tabs at Head Trim as shown on PW09024 & PW09025.



Note: All trim is to be installed BEFORE blanket insulation is applied to walls.

Note: Field measure Door Width and Height before making field cuts and adjust cut dimensions accordingly.

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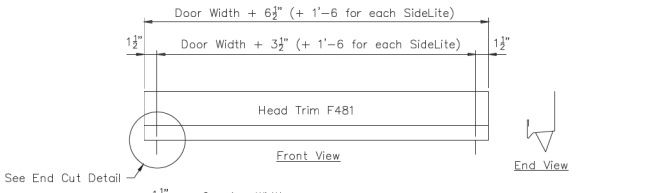
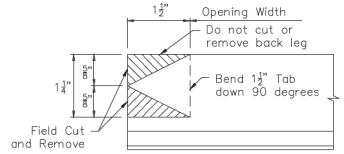
PBR Wall Panel - Walk Door And Glass-Front Walk Door - Field Notch and Bend Tabs at Head Trim

Page PW09025 | Date Mar '20 | Rev 02

Note: Trim Installation can be done by Field Notch Panel as shown on PW09022 & PW09023 OR with Field Notch and Bend Tabs at Head Trim as shown on PW09024 & PW09025.

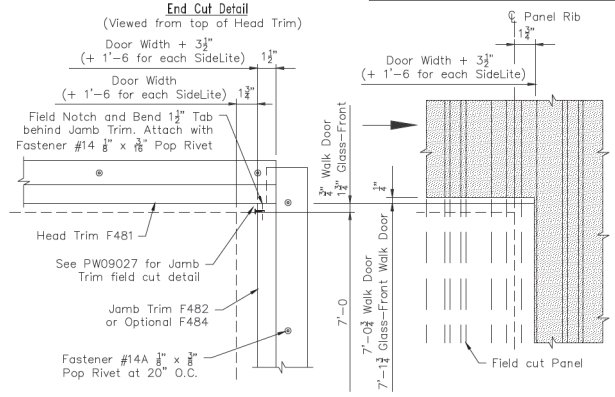




End Cut Detail
(Viewed from top of Head Trim)

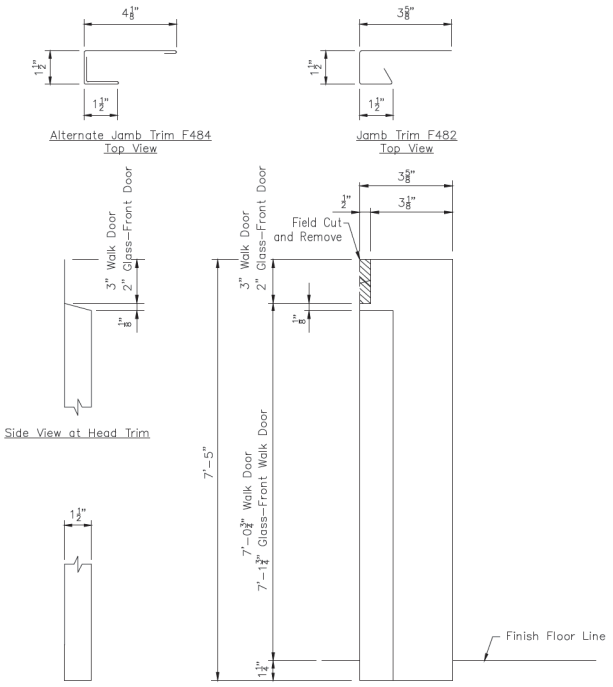
Note: All trim is to be installed BEFORE blanket insulation is applied to walls

Note: Panel position is shown with Panel Rib and Door on 1'-0 module. Location of Rib may vary depending on the Door Width and location. Field measure before cutting Panel and Trim.



PBR Wall Panel - Walk Door And Glass-Front Walk Door Jamb Trim Field Cut Details

Page PW09027 | Date Mar '20 | Rev 03

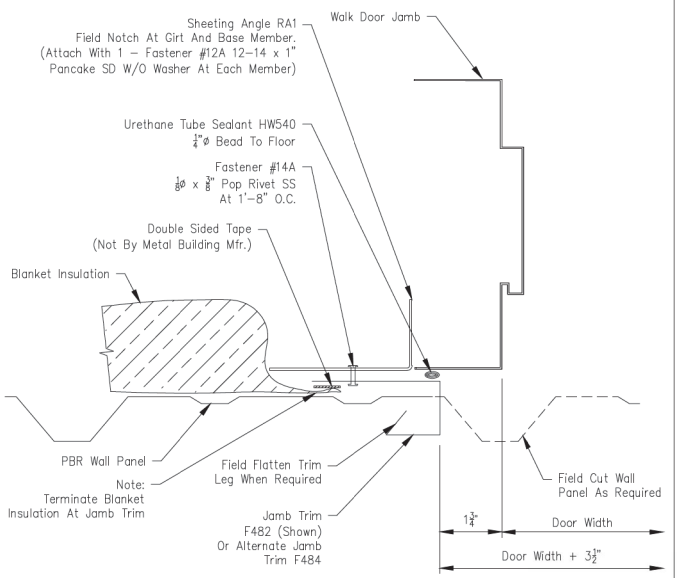


Note: Field measure Door Height before making field cuts and adjust cut dimensions accordingly so that Jamb Trim fits to Head Trim & at 1¼" below Finish Floor Line.

Jamb Trim F482 and Alternate Jamb Trim F484
Front View
Right Jamb Trim as shown
Left Jamb Trim opposite hand

PBR Wall Panel - Knock Down Door Jamb Trim Installation

Page PW09031 | Date Mar '20 | Rev 07



Alternate Jamb Trim F484

Note:
All Trim Is To Be Installed BEFORE Blanket Insulation Is Applied To Walls.

Location Of Panel Rib May Vary Depending On Door Width, Door Location And Panel Starting Location. Refer To Erection Drawings For Panel Layout. Field Measure Before Cutting Panel And Trim.

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PBR Wall Panel - Knock Down Walk Door - Head Trim Installation

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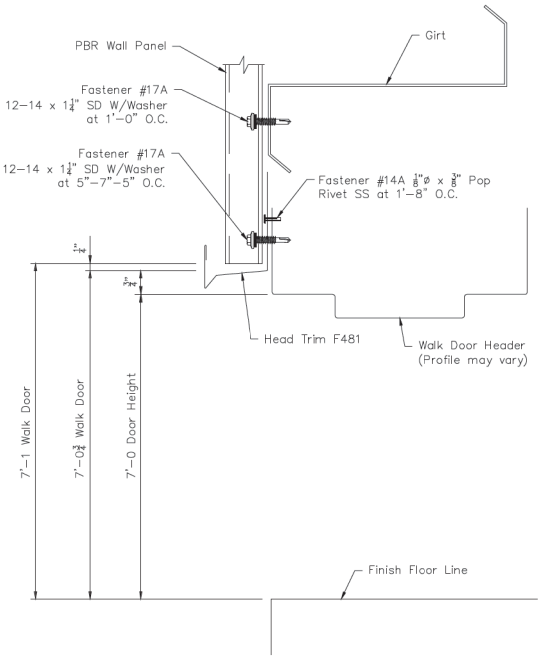
Date	Rev
May '19	03



Note: All trim is to be installed BEFORE blanket insulation is applied to walls.

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	2/ 3/23	FOR ERECTOR INSTALLATION	IES	IES	SXV

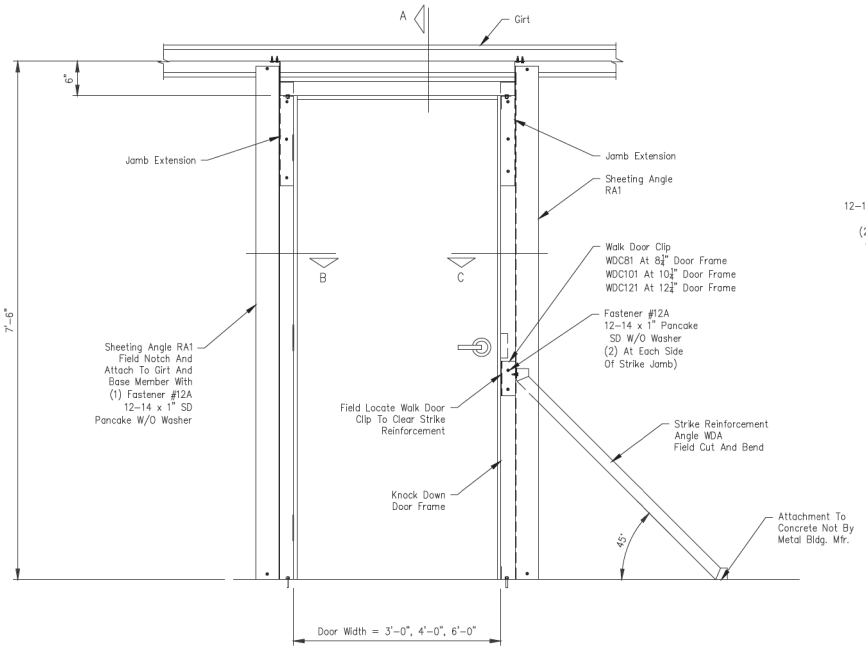
CECO Building Systems
Columbus, MS. (662) 328–6722
Mount Pleasant, IA. (319) 385–8001
Rocky Mount, NC. (252) 977–2131
www.cecobuildings.com

PROJECT: NATALIE DAVIDSON
CUSTOMER: THE STEEL BUILDER
OWNER: NATALIE DAVIDSON
LOCATION: SPARKS,NV 89441–8549 US

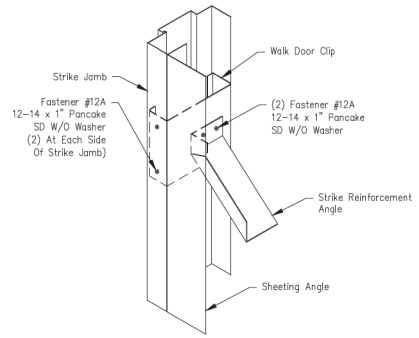
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	2/ 3/23	N.T.S.	1	A	19–B–27926	DET23	0

Drawing has been digitally signed.

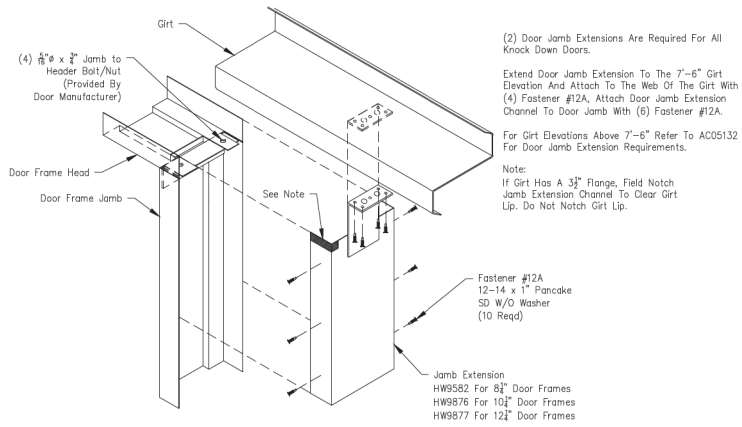
PROFESSIONAL ENGINEER–STATE OF NEVADA
BEJUN N. ANKLESARIA
Exp: 12/31/24
STRUCTURAL
Feb 07, 2023
No. 17245



Door Elevation



Walk Door Clip/Strike Reinforcement Angle Isometric



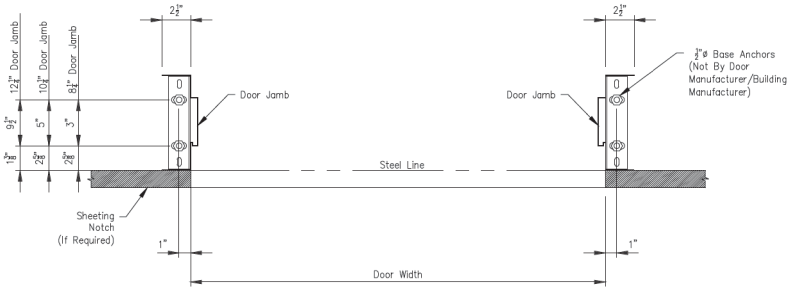
(2) Door Jamb Extensions Are Required For All Knock Down Doors.

Extend Door Jamb Extension To The 7'-6" Girt Elevation And Attach To The Web Of The Girt With (4) Fastener #12A, Attach Door Jamb Extension Channel To Door Jamb With (6) Fastener #12A.

For Girt Elevations Above 7'-6" Refer To AC05132 For Door Jamb Extension Requirements.

Note:
If Girt Has A 3½" Flange, Field Notch Jamb Extension Channel To Clear Girt Lip. Do Not Notch Girt Lip.

Door Jamb Extension Isometric

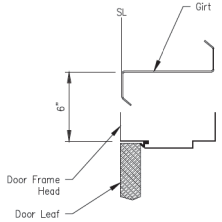


The Adequacy Of The ½"ø Base Anchor Is Not The Responsibility Of The Building Manufacturer. The Adequacy Of These Base Anchors Should Be Determined By A Qualified Foundation Engineer.

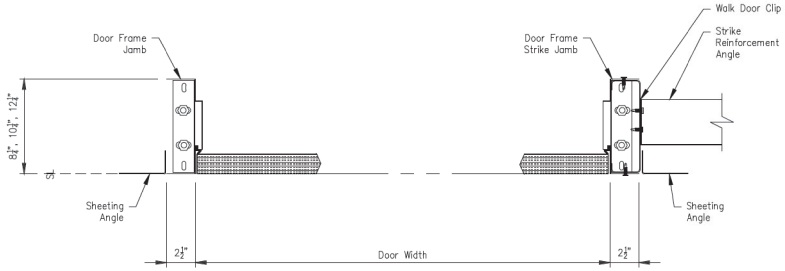
Verify Door Jamb Base Clip Dimensions With Patterns Shown Prior To Placement Of Door Anchors And Adjust Patterns If Needed.

Note: 12¼" Frames May Not Have Kerf Door Frame Feature Depending On Door Manufacturer.

Knock Down Door Anchor Placement



Section A



Section B

Section C

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	2/ 3/23	FOR ERECTOR INSTALLATION	IES	IES	SXV

CECO Building Systems
Columbus, MS. (662) 328-6722
Mount Pleasant, IA. (319) 385-8001
Rocky Mount, NC. (252) 977-2131
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PROJECT:	NATALIE DAVIDSON
CUSTOMER:	THE STEEL BUILDER
OWNER:	NATALIE DAVIDSON
LOCATION:	SPARKS,NV 89441-8549 US

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
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PROFESSIONAL ENGINEER - STATE OF NEVADA
BEJUN N. ANKLESARIA
Exp: 12/31/24
STRUCTURAL
Feb 07, 2023
No. 17245

Knock Down Door - Girt At 7'-6" Without Low Girt

Page	
AC05200	
Date	Rev
Nov '18	00

Field Service Procedures

In Order To Give You Prompt Services And Keep Problems To A Minimum, Please Handle Any Shortages Or Back Charges In The Following Manner:

1. Carefully Check Your Packing List While Unloading.
2. Mark Any Items Which Appear To Be Missing And Notify The Field Service Department At The Number Shown In The Title Block As Soon As Possible. Calling Someone Else Could Delay The Proper Response.

INITIAL CLAIM:
In The Event Of An Error, The Customer Must Promptly Make A Written Or Verbal "Initial Claim" to The Manufacturer For The Correction Of Design, Drafting, Bill Of Materials Or Fabrication Error.
The "Initial Claim" Includes:

1. Description Of The Nature And Extent Of The Errors, Including Quantities.
2. Description Of The Nature And Extent Of Proposed Corrective Work, Including Estimated Man-Hours.
3. Materials To Be Purchased From Other Than the Manufacturer, Including Estimated Quantities and Cost.
4. Maximum Total Cost Of Proposed Corrective Work And Materials To Be Purchased From Other Than The Manufacturer.

SHORT MATERIALS:
Immediately Upon Delivery Of Materials, Quantities Are To Be Verified By The Customer Against Quantities That Are Billed On The Shipping Documents. Neither The Manufacturer Nor The Carrier Is Responsible For Material Shortages Against The Quantities Billed On The Shipping Documents If Such Shortages Are Not Noted On The Shipping Documents When The Material Is Delivered And Acknowledged By The Carrier's Agent. If The Carrier Is The Manufacturer, Claims For Shortages Are To Be Made By The Customer To The Common Carrier. If The Material Quantities Received Are Correct According To The Quantities Billed On The Shipping Documents, But Are Less Than The Quantities Ordered Or The Quantities That Are Necessary To Complete The Metal Building According To The Order Documents, Claim Is To Be Made To The Manufacturer.

DAMAGED OR DEFECTIVE MATERIAL:
Damaged Or Defective Material, Regardless Of The Degree Of Damage, Must be Noted On The Shipping Documents By The Customer And Acknowledged By The Carrier's Agent. The Manufacturer Is Not Responsible For Material Damaged In Unloading Of Packages Or Nested Materials, Including, But Not Limited To: Fasteners, Sheet Metal, "c" And "z" Sections And Covering Panels That Become Wet And/Or Damaged By Water While In The Possession Of Others. Packaged Or Nested Material That Become Wet In Transit Must Be Unpacked, Unstacked And Dried By The Customer. If The Carrier Is The Manufacturer, The Customer Must Make Claim For Damaged Directly To The Manufacturer. If The Carrier Is A Common Carrier, The Customer Must Make The Claim For Damage To The Common Carrier. The Manufacturer Is Not Liable For Any Claim Whatsoever Including, But Not Limited To Labor Charges Of Consequential Damages Resulting From Customer's Use Of Damaged Or Defective Materials That Can Be Detected By Visual Inspection.

EXCESSIVE MATERIAL:
The Manufacturer Reserves The Right To Recover Any Material Delivered In Excess Of Those Required By The Order Documents.

OIL CANNING IS NOT A CAUSE FOR REJECTION

Authorization For Corrective Work

Normal Erection Operations Include The Correction Of Minor Misfits By Amounts Of Reaming, Chipping, Welding Or Cutting And The Drawing Of Elements Into Line Through The Use Of Drift Pins. Errors That Cannot Be Corrected By The Foregoing Means Or Which Require Major Changes In The Member Configuration Should Be Reported Immediately To The Owner And The Fabricator By The Erector, To Enable Whoever Is Responsible Either To Correct The Error Or Approve The Most Efficient And Economical Method Of Correction To Be Used By Others. (AISC 303-10, Section 7.14). If The Error Is The Fault Of The Manufacturer An "Authorization For Corrective Work" Must Be Issued In Writing By The Manufacturer To Authorize The Corrective Work At A Cost Not To Exceed The Maximum Total Cost Set Forth. Alternative Corrective Work Other Than That Proposed In The "Initial Claim" May Be Directed By The Manufacturer In The "Authorization Of Corrective Work". Only The Field Service Department May Authorize Corrective Work.

FINAL CLAIM:
The "Final Claim" In Writing Must Be Forwarded By The Customer To The Manufacturer Within (10) Days Of The Completion Of The Corrective Work Authorized By The Manufacturer.

THE "FINAL CLAIM" MUST INCLUDE:

1. Actual Number Of Man-Hours By Dated Of Direct Labor Use On Corrective Work And Actual Hourly Rate Of Pay.
2. Taxes And Insurance On Total Actual Direct Labor.
3. Other Direct Costs On Actual Direct Labor.
4. Cost Of Materials (Not Minor Supplies) Authorized By The Manufacturer To Be Purchased From Other Than The Manufacturer, Including Copies Of Paid Invoices.
5. Total Actual Direct Cost Of Corrective Work (Sum Of 1, 2, 3, And 4). The "Final Claims Are Credited To The Customer By The Manufacturer In The Amount Not To Exceed The Lesser Of The Maximum Total Cost Set Forth In The "Authorization For Corrective Work" Or The Total Direct Cost Of Corrective Work.

** IMPORTANT NOTE **
Cost Of Equipment (Rental Or Depreciation), Small Tools, Supervision, Overhead And Profit Are Not Subjected To Claims.

SHIPMENT ARRIVAL TIME:
Every Effort Will Be Made To See That The Carrier Arrives At The Jobsite On The Requested Hour. Manufacturer Makes No Warranty And Accepts No Responsibility For Costs Associated With A Shipment Not Arriving At The Requested Time Unless A Separate Agreement Has Been Made In Writing For A Guaranteed Arrival Time.

Unloading, Handling And Storage

STRUCTURAL:
A Great Amount Of Time And Trouble Can Be Saved If The Building Parts Are Unloaded At The Building Site According To A Pre-Arranged Plan. Proper Location And Handling Of Components Will Eliminate Unnecessary Handling.

NOTE:
Piece Marks Are Stenciled On The Primary Structural Members At The Lower End, 1'-0" From The End. Inspect All Shipments Prior To Releasing The Tie-downs For Loads That May Have Shifted During Transit.

REMEMBER SAFETY FIRST:
Blocking Under Columns And Rafters Protect The Splice Plates And The Slab From Damage During The Unloading Process. It Also Facilitates The Placing Of Slings And Cables Around Members For Later Lifting And Allows Members To Be Bolted Together Into Sub-assemblies While On The Ground. Extra Care Should Always Be Exercised In The Unloading Operation To Prevent Injuries From Handling Steel And To Prevent Damage To Materials And The Concrete Slab. If Water Is Allowed To Remain For Extended Periods In Bundles Of Primed Parts Such As Girts, Purlins, Etc., The Pigment Will Fade And The Paint Will Gradually Soften Reducing Its Bond To The Steel. Therefore, Upon Receipt Of A Job, All Bundles Of Primed Parts Should Be Stored At An Angle To Allow Any Trapped Water To Drain Away And Permit Air Circulation For Drying. Puddles Of Water Should Not Be Allowed To Collect And Remain On Columns Or Rafters For Same Reason.

The Coat Of Shop Primer Is Intended To Protect The Steel Framing Only For A Short Period Of Exposure To Ordinary Atmospheric Conditions. The Coat Of Shop Primer Does Not Provide The Uniformity Of Appearance, Or The Durability And Corrosion Resistance Of A Field Applied Finish Coat Of Paint Over Shop Primer.

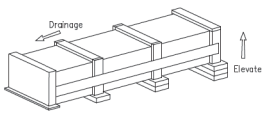
Roof And Wall Panels

Manufacturer's Roof And Wall Panels Include Color Coated, Galvalume, And Galvanized, Provide Excellent Service Under Widely Varied Conditions. All Unloading And Erection Personnel Should Fully Understand That These Panels Are Quality Merchandise, Which Merits Cautious Care And Handling.

UNDER NO CIRCUMSTANCES SHOULD PANELS BE HANDLED ROUGHLY
Packages Of Sheets Should Be Lifted Off The Truck With Extreme Care Taken To Ensure That No Damage Occurs To Ends Of The Sheets Or to Side Ribs. The Packages Should Be Stored Off The Ground Sufficiently High To Allow Air Circulation Underneath The Packages. This Avoids Ground Moisture And Deters People From Walking On The Packages. One End Of The Package Should Be Elevated To Encourage Drainage In Case Of Rain. The Manufacturer Exercises Caution During Fabrication An Shipping Operations To Ensure That All Panel Stock Is Kept Dry. However Due To Climatic Conditions, Water Formed By Condensation Of Humid Air Become Trapped Between Sheets. Water Can Also Be Trapped Between The Stacked Sheets When Exposed To Rain. This May Discoloration Caused By Trapped Moisture. The Stain Is Usually Superficial And Has Little Effect On The Appearance Or Service Life Of The Panels As Long As It Not Permitted To Remain On The Panel. However, Moisture In Contact With The Surface Of The panel Over An Extended Period Can Severely Attack The Finish And Reduce The Effective Service Life. See R1-07 Titled "Damage From Condensation Or Trapped Water".

CAUTION:
Care Should Always Be Taken When Walking On Panels. Use Safety Lines And Net When Necessary. Panels Are Slippery, Wipe Dry Any Moisture Or Surface Material That Has Puddle From Bundles Stored On A Slope. Dew, Frost, Or Other Forms Of Moisture Greatly Increase The Slipperiness Of The Panels. Always Assume Panel Surface Is Slippery And Act Accordingly. Never Walk Of Step On Skylights Or Translucent Panels.

Use Wood Blocking To Elevate And Slope The Panels In A Manner That Allows Moisture To Drain. Wood Blocking Placed Between Bundles Will Provide Additional Air Circulation. When Handling Or Uncrating The Panels, Lift Rather Than Slide Them Apart. Burred Edges May Scratch The Coated Surfaces When Sheets Are Slid Over One Another. Never Allow Panels To Be Walked On While On The Ground.



Roof And Wall Panel Damage During Construction

The Quality Of Workmanship In Steel Construction Practices And Handling Methods Used During The Construction Of The Metal Building Can Significantly Affect The Appearance And Performance Of The Building Panels. Panel Damage During Construction Can Be The Result Of Faulty Installation Methods And/or Carelessness.

Overdriven Fasteners Cause Indentations Or Shallow Pockets In The Panel Around The Fastener Head. Rain Water Or Condensation Moisture Combined With Atmospheric Pollutants (principally Sulfur Dioxides) And Dirt Particles Collect In These Pockets. The Combination Of Pollutants And Water Creates Acid Solutions That Will Cause Corrosion Damage To The Panel And Fastener. Rain May Wash Some Pollutants Away, But Moisture In Form Of High Humidity Can Keep These Areas Wet And Continue The Problem. Overdriving The Fastener Also Forces The Sealing Washer From Under The Head Creating A Leak At This Point. Proper Torque Adjustment Of The Screw Gun Or Preferably The Use Of A Depth Gauge Will Eliminate The Problem Of Overdriven Fasteners.

It Is Extremely Important That All Drill Shavings From The Installation Of Panel Fasteners And Fillings From The Saw Cutting Of Panels Be Removed From The Panel Surface. Corrosion Can Occur In A Matter Of Hours When These Shavings Or Fillings Are Not Removed And Are In Contact With Water Or Condensed Moisture. When Panels Are Pre-Drilled Or Cut In The Stock Prior To Erection All Shavings Must Be Cleaned From Both Sides Of The Panel To Prevent Corrosion Of The Panel By These Particles. It Is Imperative That The Roof Be Swept Clean At Least Daily And Certainly At Job Completion. The Final Cleaning Of The Roof Should Be Done Prior To Installing The Gutter So That The Shavings Are Not Deposited Into The Gutter And Left To Corrode. Any Other Foreign Objects Or Debris Left By Construction Personnel Should Also Be Removed From The Roof During The Erection Of The Roof And The Installation Of Such Equipment As Air Condition Units, Etc..

Personnel Walking On The Panel Can Cause Damage. Workmen Should Step Or Walk In The Broad Flat Areas Of The Panel And Avoid Stepping On The Panel Ends And Edges Which Can Be Bent By Careless Handling. If This Damage Is Severe, The Edges Must Be Straighten Prior To Erection Since The Appearance And/or Weather Tightness Of The Panel Could Be Affected. Dragging One Panel Across Another Can Cut Or Abrade The Coating Causing Unsightly Marks On The Panel Surface.

Attempts To Erect Panels During Windy Conditions Should Be Avoided To Prevent Damage And Of Safety Considerations.

Leaving Dirt Piled Against The Exterior Wall Panels At The Foundation Will Cause Panel Damage. This Dirt May Be Wet Or At Least Contain Some Moisture. Mud May Have Splashed Onto The Wall During Construction. Corrosion Damage May Occur Where This Dirt Or Mud Contacts The Panel. In Areas Where Lime Stabilization Of The Soil Is Required, Corrosion Damage From The Soil's Content Will Be Accelerated And Most Likely Be Severe. All Dirt Must Be Removed From The Panel Walls At The Time Of Completion Of Work. Pre-Painted Panels May Require Touch-up If The Coating Has Been Damaged During Handling Or Erection.

The Appearance Of The Building May Be Affected If Damaged Spots Or Scratches Are Located In Highly Visible Places Such As Around Doors, Windows, Etc.. If Damage Is Extensive Then Replacement Of The Entire Panel Should Be Considered.

Types Of Finishes

SHOP PRIMED STEEL:
All Structural Members Of The Metal Building System Not Fabricated Of Corrosion Resistant Material Or Protected By A Corrosion Resistant Coating Are Painted With One Coat Of Shop Primer Meeting The Performance Requirements Of SSPC Paint Specification No.15. The Coat Of Shop Primer Is Intended To Protect The Steel Framing For Only A Short Period Of Exposure To Ordinary Atmospheric Conditions. Shop Primed Steel Which Is Stored In The Field Pending Erection Should Be Kept Free Of The Ground And So Positioned As To Minimize Water Holding Pockets, Dust, Mud And Other Contamination Of The Primer Film. Repairs Of Damaged To Primed Surfaces And/or Removal Of Foreign Material Due To Improper Field Storage Or Site Conditions Are Not The Responsibility Of The Manufacturer. The Manufacturer Is Not Responsible For Deterioration Of The Shop Coat Of Primer Or Corrosion That May Result From Exposure To Atmospheric And Environmental Conditions, Nor The Compatibility Of The Primer To Any Field Applied Coating. Minor Abrasions To The Shop Coat (Including Galvanizing) Caused By Handling, Loading, Shipping, Unloading And Erection After Painting Or Galvanizing Are Unavoidable. (MBMA 2012, Chapter IV 4.2.4).

GALVALUME:
Galvalume Is The Trade Name For A Patented Steel Sheet And Coil Product Having A Coating Of Corrosion Resistant Aluminum-Zinc Alloy. The Mixture Is Balanced To Obtain The Coating That Retains The Corrosion Resistance And Heat Reflectivity Of Aluminum And Galvanic Protection Of Zinc. The Best Properties Of Both Aluminum And Zinc Are Combined In This Coating And Offer Added Service Life For The Building.

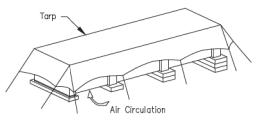
Pre-Painted:
Using Galvalume Steel As A Substrate, Pre-Painted Steel Is Given An Additional Rust Inhibitor Primer Coat. This Primer Coat Further Increases The Corrosion Resistance. These Coatings Are Applied To The Exterior Surface Of The Panels And A Wash Coat Designed Only For Interior Use, Is Applied On The Opposite Side. Galvalume And Pre-Painted Steel Can Give Excellent Service For Many Years If A Few Rules Concerning Their Care And Maintenance Are Observed. All Of These Finishes Are Equally Subject To Damage And Corrosion When Care Is Not Provided.

PAINT AND COATING MAINTENANCE:
Remove Smudge Marks From Bare Galvalume:
Formula 409 Has Proven To Be Somewhat Effective. Lightly Rub With A Clean Cloth And Rinse With Water. Do Not Rub More Than Required To Remove Smudge Marks. No Product Will Remove All Smudge Marks.
Remove Rust Stains:
Soft Scrub Without Bleach Has Proven To be Somewhat Effective. Rub With A Soft Cloth And Rinse With Water. Do Not Rub More Than Required To Remove Stain. No Product Will Completely Remove Rust Stains.
To Touch-Up Scratches In Paint (Not Bare Metal):
Clean Area To Be Painted With Mild Detergent. Rinse Thoroughly And Dry. Using A Small Artist's Brush, Lightly Apply A Minimal Amount Of Color Matched Touch-Up Pain Required To Fill/Cover The Scratch. Contact The Building Manufacturer For Assistance With Ordering/Purchasing Touch-Up Paint As Needed.

Damage From Condensation Or Trapped Water

It Is Extremely Important That The Panels Be Monitored For Evidence Or Trapped Water Or Moisture Condensation While Awaiting Erection. High Humidity Conditions With Temperature Cycling Will Cause Condensation Between Panels Within The Bundle. Condensation Can Occur Frequently Near The Sea Coast Or Other Large Bodies Of Water.

If Jobsite Covers Are Used, They Should Be Tied Away From The Bundle At Corners To Allow Air Circulation Around The Bundle. This Will Help Prevent Moisture Evaporating From The Ground Or Building Floor From Condensing On The Panels. Plastic Or Other Impermeable Covers Are Not Recommended. Immediate Action Is Required If The Panels Are Found To Be Wet From Any Cause. The Bundles Must Be Opened And Each Panel Un-Stacked And Thoroughly Dried On Both Sides. Re-Stacking The Panel At A Slight Angle To Each Other To Prevent Nesting Will Allow Air Circulation And Assist In Keeping The Panel Dry. In Severe Conditions Large Fans Can Be Used To Circulate Air Between The Un-Stacked Panels And Accelerate Drying. Damage To The panel Coating Occurs When Panels Become Wet And Are Allowed To stay wet. damage Can Occur To Nested Panels Within 24 to 48 Hours. This Damage Shows Corrosion And Discoloration Of The Panel Surface And Is Commonly Called Wet Storage. Stain, Zinc Oxidation, Or "White Rust".



A Softening Of The Paint Film Can Occur With Pre-Painted Steel Under Wet Storage Conditions And The Durability Of The Panel Finish Substantially Decrease. Bare Galvanized And Galvalume Panels React More Quickly To Surface Oxidation Since They Lack The Additional Protection Of Paint. Zinc Coated Or Galvalume Panels Under Normal Exposure Form A Zinc Aluminum Oxide Film On Their Surface Allowing A Slow Oxidation Process Called "Weathering" To Occur That Inhibits Further Corrosion. In Nested Bundles Constant Contact Of The Panels With Condensed Or Trapped Water Prevents This Weathering Process.

Rapid Oxidation Of The Zinc or Zinc Aluminum Coating Can Now Occur And May Lead To "Red Rust" In A Short Time. If Discoloration Or Stains Are Minor A Household Cleaner Of The Type Used On Porcelain Sinks And Bathtubs May Be Used To Remove Stains. Wire Brushing Or Abrasive Materials Should be Avoided Since Scratching Or Removal Of The Coating Could Occur. Panel With Significant Damage Should Be Replaced By The Buyer Prior To Erection.

Safety Commitment

The Builder/Contractor Is Responsible For Applying And Observing All Pertinent Safety Rules And OSHA Standards As Applicable.

The Building Manufacturer Has A Commitment To Manufacture Quality Building Components That Can Be Safely Erected. However The Safety Commitment And Job Site Practices Of The Erector Are Beyond The Control Of The Building Manufacturer.

It Is Strongly Recommended That Safe Working Conditions And Accident Prevention Practices Be The Top Priority Of Any Job Site.

Local, State And Federal Safety And health Standards, Whether Standard Statuary Or Customary, Should Always Be Followed To Help Ensure Worker Safety.

Make Sure All Employees Know The Safest And Most Productive Way Of Erecting A Building. Emergency Procedures Should Be Known To All Employees. Daily Meetings Highlighting Safety Procedures Are Also Recommended. The Use Of Hard Hats, Rubber Sole Shoes For Roof Work, Proper Equipment For Handling Material And Safety Nets Where Applicable Are Recommended

For The Purposes Of Determining Lift Requirements, No Bundle Supplied By The Manufacturer Will Exceed 4,000 Pounds. For Further Information Also reference The Bill Of Materials For Individual Member Weights Of Structural Members. If Additional Information Is Required Contact The Field Service Department.

ICE AND SNOW REMOVAL:
Excessive Ice And Snow Removal Should Be Removed From The Roof Immediately To Prevent Damage To Roof And Possible Collapse. Do Not Use Metal Tools To remove The Ice Or Snow As This Can Damage The Paint And/Or Galvalume Coatings. Also Be Careful Around Pipes And Flashing's.
Be Extremely Careful If Your Roof Has Light Transmitting Panels. These Panels Will Not Support A Person's Weight And Will Be Difficult Or Impossible To See If They Are Covered With Ice Or Snow. See MBMA Low-Rise Building Systems Manual, Appendix AB For Details On Snow Removal Procedures. These Procedures Should Commence When Half Of The Design Roof Snow Load Is Realized.

DEBRIS REMOVAL:
Any Foreign Debris Such As Sawdust,Dirt, Leaves, Animal Droppings, Etc. Will Cause Corrosion Of The Roof, Gutters, Trim, Etc. If Left On The Building Surface For A Long Enough Time. The Roof Should Be Periodically Inspected For Such Conditions And If Found, They Should Be Rectified In A Manner Consistent With These Roof Maintenance Guidelines. Never Allow Treated Lumber Or Concrete/Mortar/Grout To Come In Contact With Roof Panels, Especially Galvalume For Extended Periods Of Time.

PERIODIC INSPECTION:
All High-Strength Shall Be Periodically Be Inspected For Tightness. Particularly In Crane Buildings And After Seismic Or Wind Activity. The Crane Manufacturer Will Specify A Minimum Period But It Should Not Exceed Two Years.

DRAINAGE:

1. Keep Roof Free Of Debris And Keep Debris Out Of Gutter To Allow Water Quickly Drain From The Roof.
2. Do Not Use Wood Blocking To Hold Equipment Off The Panel Seams. This Blocks The Flow Of Water And Hold Moisture.
3. Do Not Allow Rooftop AC Units Or Evaporative Coolers To Drain Onto The Roof.
4. Anything That Traps Or Holds Moisture On A Roof Will Cause Premature Corrosion.

Roof Maintenance Guidelines

1. Inspect Roof For Damage After Heavy Storms.

2. Inspect And Reseal As Necessary All Roof Curbs And Other Penetrations With Urethane Sealant.

3. Always Get Manufacturer Approval Before Making Any Modifications To The Roof.

4. Repaint Any Areas That Are Susceptible To Rust As Required.

5. When Performing Roof Maintenance, Always Take The Following Precautions:
   a. Use Fall Protection And Other Safety Protection As Required.
   b. Do Not Walk On Roof Flashing Such As Gutter, Rake, Hip Or Ridge Flash.
   c. Do Not Walk On Light Transmitting Panels (LTP's). They Will Not Support A Person's Weight.
   d. Guard All LTP's And Roof Openings.
   e. Step Only In The Panel Flat Directly On Or In Close Proximity To A Supporting Roof Structural.

6. After Other Trades Have Been On The Roof For Any Reason, Inspect The Roof For Damage Caused By Workers Including Chemical Or Solvent Spills, Scratches In The Paint Or Galvalume Coating, Excessive Foot Traffic And Punctures. Make Sure That All Debris Or Scrap Left Behind By Workers Is Removed From The Roof Immediately. Avoid Using Cutoff Saws And Welding Equipment Over The Roof. The Roof Must Adequately Protected.

FOOT TRAFFIC:
Keep Foot Traffic To A Minimum. Heavy Foot Traffic Can Cause Ponding On Low Pitched Roofs. This Is Particularly True Just Upslope From The Eave And At Endlaps.
Always Walk In The Flat Of The Panel Near A Supporting Roof Structural. Do Not Walk On Trim Or In Gutters.
On Bare Galvalume Roofs, Excessive Foot Traffic May Cause Black Burnish Marks. If Regular Foot Traffic Is Planned For A Roof, Provisions Should Be Made For A Properly Designed And Installed Walkway System. In Order To Limit Access To The Roof, Roof Hatches Or Access Ladders Should Be Locked At All Times. A Sign Posted At The Access Site Stating That Only Authorized Personnel Are Allowed On The Roof. In Addition A Log Book Should Be Kept Of All Visits To The Roof And The Reason For Such Visits.

DISSIMILAR METALS:
Never Allow Your Roof To Come In Contact With, Or Water Runoff From Any Dissimilar Metal Including But Not Limited To:
Copper, Lead Or Graphite, This Includes Copper And Arsenic Salts Used In Treated Lumber, Calcium Used In Concrete, Mortar And Grout.

Never Step On Light Transmitting Panels (LTP's) Or Unattended Roof Panels

Panels May Collapse
If Not Properly Secured

Roof Panels Must Be Completely Attached To The Purlins And To Panels On Either Side Before They Can Be A Safe Walking Surface. Light Transmitting Panels LTP's) Translucent Panels Can Never Be Considered As A Walking Surface.

Partially Attached Or Unattached Panels Should Never Be Walked On!

Do Not:

1. Step On Rib At Edge Of Panel.

2. Step Near Crease In Rib At Edge Of Panel.

3. Step Within 5 Feet Of Edge On Unsecured Panel.

A Single Roof Panel Must Never Be Used As A Work Platform. An OSHA Approved Runway Should Be Used For Work Platforms. (Consult OSHA Safety And Health Regulations For The Construction Industry). Safety First!

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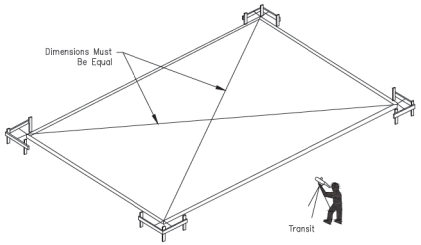
Building Anchorage

1. To Determine That The Foundation Is Square, Measure Diagonal Dimensions To Be Sure They Are Of Equal Length.
2. To Determine That The Foundation Is Level, Set Up A Transit Or Level And Use A Level Rod To Obtain The Elevation At All Columns.
3. Carefully Check The Location Of All Anchor Rods Against The Anchor Rod Setting Plan Furnished By The Manufacturer. All Dimensions Must Be Identical To Assure A Proper Start-up.

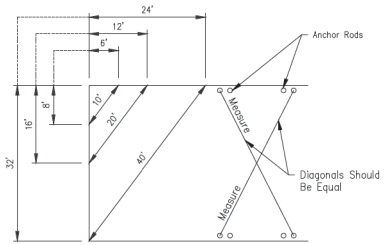


Pre-Erection Notes:

The Following Notes, Procedures And Suggested Recommendations Are Important Parts Of The Pre-Erection Process.

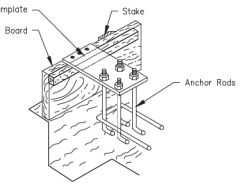
1.) Prior To The Time The Erection Crew Arrives, A Responsible Person Should Check The Job Site For Foundation Readiness, Square, And Accuracy And Anchor Rod Size And Location.

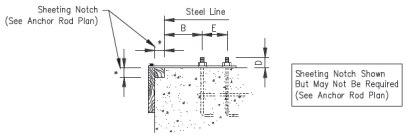
The Drawing Shown Below Indicates A Method Which May Be Used To Check The Foundation And Bolts For Square.



Measure Along Adjacent Sides Of Foundation Using A Pair Of Dimensions Shown. If The Diagonal Distance Between These Points Is As Noted, The Corner Is Square. Diagonal Measurements Between Opposite Anchor Rods Will Indicate If These Bolts Are Set Square.

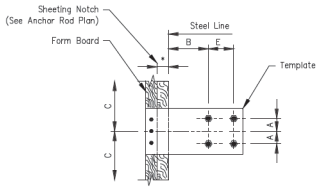
It Is Extremely Important That Anchor Rods Are Placed Accurately And In Accordance With The Anchor Rod Setting Plan. All Anchor Rods Should Be Held In Place With A Template Or Similar Means, So That They Will Remain Plumb And In Correct Location During The Placement Of The Concrete. A Final Check Should Be Made After Completion Of The Concrete Work And Prior To The Steel Installation. This Will Allow Necessary Corrections To Be Made Before Costly Installation Labor And Equipment Arrives.





Sheeting Notch Shown But May Not Be Required (See Anchor Rod Plan)

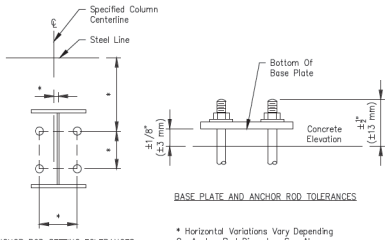
Projection Of Anchor Rods (D) Given On Anchor Rod Plan



Dimensions A, B, And C Given On Anchor Rod Plan

AISC Code Of Standard Practice For Steel Building And Bridges Tolerances For Setting Anchor Rods

Anchor Rod Diameter, Inches (mm)	*Horizontal Variation, Inches (mm)
¾" and ⅞" (19 And 22 mm)	¼" (6 mm)
1", 1¼", 1½" (25, 31, 38 mm)	⅜" (10 mm)
1¾", 2", 2¼" (44, 50, 63 mm)	½" (13 mm)



ANCHOR ROD SETTING TOLERANCES

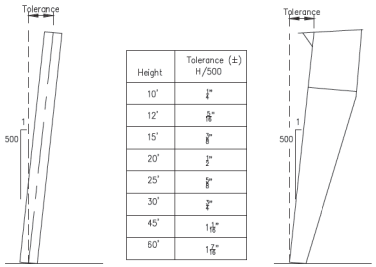
BASE PLATE AND ANCHOR ROD TOLERANCES

* Horizontal Variations Vary Depending On Anchor Rod Diameter. See Above

Erection Tolerances

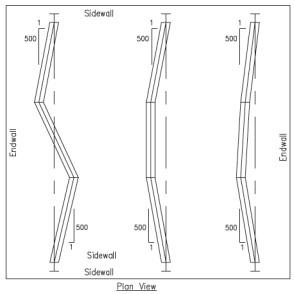
ERECTION BRACING:
It Is The Responsibility Of The Erector To Determine, Furnish And Install All Temporary Supports Such As Temporary Guys, Beams, Falsework, Cribbing, Or Other Elements Required For The Erection Operation (In Accordance With Section 7.10.3 Of ANSI/AISC 303, Code Of Standard Practice For Steel Building And Bridges).

COLUMN ALIGNMENT TOLERANCES



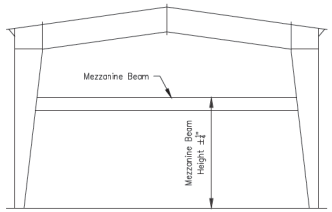
Height	Tolerance (±) H/500
10'	1/4"
12'	5/16"
15'	3/8"
20'	1/2"
25'	5/8"
30'	3/4"
45'	1 1/16"
60'	1 7/16"

ALIGNMENT TOLERANCE FOR MEMBERS WITH FIELD SPLICES



Plan View

MEZZANINE BEAM HEIGHT TOLERANCE



General Erection Notes

1.) All Structural Framing Members, Purlins, Girts, Clips, Flange Braces, Bolts, Bracing Systems, Roof And Wall Panels, Etc. Must Be Installed As Shown On Erection Drawings.

2.) It Is Extremely Important, Especially During Construction, That Panels At The Eaves, Rakes And Ridges Be Kept Secure.

Panel Cautions And Notes

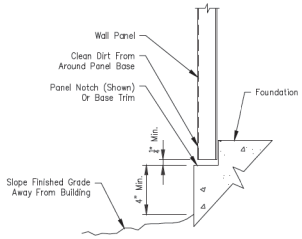
To Minimize Potential Of Corrosive Action At The Bottom Edge Of Wall Panels, The Contractor Must Assure That The Following Procedures Are Followed:

1.) The Concrete Foundation Should Be Cured For A Minimum Of Seven (7) Days Before Wall Panels Are Installed. (Uncured Concrete Is Highly Alkaline And Metal Panels Can Undergo Varying Degrees Of Corrosive Attack When In Direct Contact With The Concrete.) After The First Week Of The Curing Cycle, The Reaction Between Metallic Coatings On Steel And The Concrete Is Essentially Halted.

2.) Top Of Finish Grade At Building To Be A Minimum Of Four (4) Inches Below Bottom Of Panel.

3.) Finish Grade Is To Slope Away From Building To Ensure Proper Drainage.

4.) Upon Completion Of Finish Grading, All Dirt Is To Be Cleaned From Around Base Of Wall Panel Where It May Have Collected In Panel Notch Or On Base Trim.



Fastener Installation

Correct Fastener Installation Is One Of The Most Critical Steps When Installing Roof/Wall Panels. Drive The Fastener In Until It Is Tight And The Washer Is Firmly Seated. Do Not Overdrive Fasteners.
A Slight Extrusion Of Neoprene Around The Washer Is A Good Visual Tightness Check. Always Use The Proper Tool To Install Fasteners. A Fastener Driver (Screw Gun) With A RPM Of 1700-2000 Should Be Used For Self-Drilling Screws. A 500-600 RPM Fastener Driver Should Be Used For Self-Tapping Screws. Discard Worn Sockets, These Can Cause The Fastener To Wobble During Installation.

Note: Always Remove Metal Filings From Surface Of Panels At The End Of Each Work Period. Rusting Filings Can Destroy The Paint Finish And Void Any Warranty.

Correct Compression Of Sealing Washer | Too Tight Compression Of Sealing Washer | Too Loose Compression Of Sealing Washer

Tape And Tube Sealant

Proper Tape And Tube Sealant Application Is Critical To The Weather Tightness Of A Building. Tape Sealant Should Not Be Stretched When Installed. Apply Only To Clean, Dry Surfaces. Keep Only Enough Sealants On The Roof That Can Be Installed In A Day. During Warm Weather, Store Sealants In A Cool Dry Place. During Cold Weather (below 60°) Sealants Must Be Kept Warm (60°-90°) Until Application. After Tape Sealant Has Been Applied, Keep Protective Paper In Place Until Panel Is Ready To Be Installed.

Important Note

All Details, Recommendations And Suggestions Contained In This Erection Guide Of This Drawings Set Are For General Guidelines Only, And Not Meant To Be All-Inclusive. Industry Accepted Installation Practices With Regard To All Areas Not Specifically Discussed In This Section Should Be Followed. Only Experienced, Knowledgeable Installers Familiar With Accepted Practices Should Be Used To Assure A Quality Project.

It Is Emphasized That The Manufacturer Is Only A Manufacturer Of Metal Building Components And Is Not Engaged In The Installation Of Its Products. Opinions Expressed By The Manufacturer About Installation Practices Noted In The Erection Guide Are Intended To Represent Only A Guide. Both The Quality And Safety Of Installation And The Ultimate Customer Satisfaction With The Completed Building Are Determined By The Experience, Expertise, And Skills Of The Installation Crews, As Well As The Equipment Available For Handling The Materials. Actual Installation Operations, Techniques And Site Conditions Are Beyond The Manufacturers Control.

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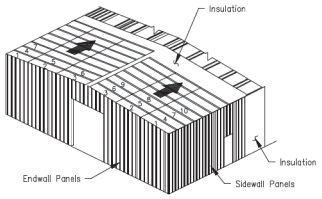
PBR Roof Panels

For PBR Roofs With Ridge Panels, It Is Recommended That Both Sides Of The Ridge Be Sheeted Simultaneously. This Will Keep The Insulation Covered For The Maximum Amount Of Time And The Panel Ribs Can Be Kept In Proper Alignment For The Ridge Panel. This Is Critical On The PBR Panels So That The Ridge Caps Can Be Properly Installed. Check For Proper Coverage As The Sheeting Progresses.



Install The First Run Of Roof Panels Across The Building From Eave To Eave Or Eave To Ridge. To Allow Proper Installation Of The Rake Trim, The Starting Location For The First Panel Must Be As Shown In The Rake Details Included With The Erection Drawings. When The First Run Is Properly Located And Aligned With The Correct Endlaps And Eave Overhangs, Fasten To Purlins. Roof Panels Should Be Installed So That The Sidelap Is In A Direction Away From Prevailing Wind. Refer To Appropriate Lap Details Included With The Erection Drawings.

Install Remaining Roof Insulation And Panels. To Avoid Accumulative Error Due To Panel Coverage Gain Or Loss, Properly Align Each Panel Before It Is Fastened. Occasional Checks Should Be Made To Ensure That Correct Panel Coverage Is Maintained. Special Attention Should Be Given To Fastener, Sealant and Closure Requirements. Refer To Details Included With The Erection Drawings.

At Finishing End Of Roof, The Last panels May Require Field Modification For Installation Of Rake Trim. Refer To Rake Details Included With The Erection Drawings. DO NOT BACK LAP THROUGH FASTENED ROOF PANELS.

NOTE: Roof Types And Installation Requirements Will Vary. Refer To The Appropriate Details For Specific Panel Used.

IMPORTANT: Loose Fasteners, Blind Rivets, Drill shavings, Etc., Must Be Removed From The Roof To Guard Against Corrosion.

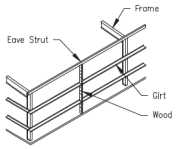
Wall Panels

Proper Horizontal And Vertical Alignment Of Supporting Structure (Girts Or Other Framing) Is The Responsibility Of The Installer. Failure To Align The Secondary members Properly Prior To Wall Installation Can Have A Direct Impact On The Final Appearance And Performance Of The Installed Wall System For Which The Metal Building Manufacturer Is Not Responsible.

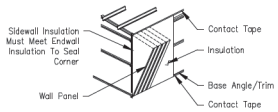
Before Installing Wall Panels, The Girts Must Be Aligned To A Level Position So That There Is No Visible Sag. This Should Be Done Directly Ahead Of Panel Installation.

Girt Leveling May Be Accomplished By Standing A Section Of Gable Angle Vertically Against The Outside Girt Flanges At Approximate Mid-bay Location. When Girts Are Level, Attach The Girt Flanges To The Angle With Vise Grip Pliers Or Temporary Screws. Wood Blocking Cut To Fit The Spaces May Also Be Used For Alignment.

Note:
Temporary Girt Blocking Is Not Recommended On Concealed Fastener Panels. The Removal Of The Blocks After Panel Installation Can Cause Oil Canning.



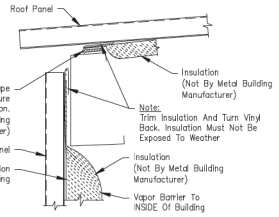
Note:
Wall Panel Type And Installation Details Will Vary. Refer To The Erection Drawings And Details For The Specific Panel Used For Your Building.



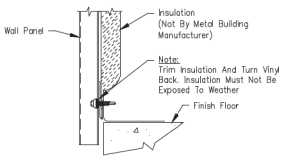
If Walls Are To Be Insulated With Blanket Insulation Over Girt Girt Flanges, Base And Eave, Place A Continuous Run Of Contact Tape Along The Eave Strut And Base Member.

Note:
At The Base, Cut Off The Insulation A Minimum Of 2" Above The Bottom Of The Wall Panel. This Will Prevent The Insulation From Hanging Below The Wall Panel And Wicking Moisture.

Note:
Additional Insulation May Be Required To Fill The Eave Strut And Prevent Condensation In Certain Climate Regions



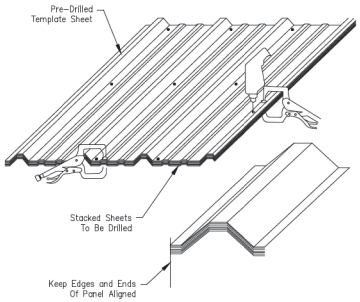
Eave Detail
(See Erection Drawings)



Base Detail
(See Erection Drawings)

Sidewall Panels Should Be Installed So That The Panel Sidelap Is In A Direction Away From The Prevailing Wind. Refer To Appropriate Lap Detail Included With Erection Drawings.)

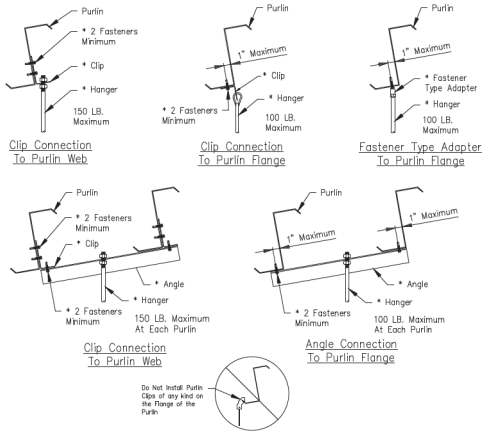
Note:
Check Periodically To Ensure That All Panels Are Aligned And Plumb.



Screw Alignment Panel
(Through Fastened Panel Only)

Note:
After Drilling Panels, It Is Important To Clean Metal Filings Off All Panel Surfaces, Including Between Panels That Are Not Installed That Day, To Avoid Rust Stains.

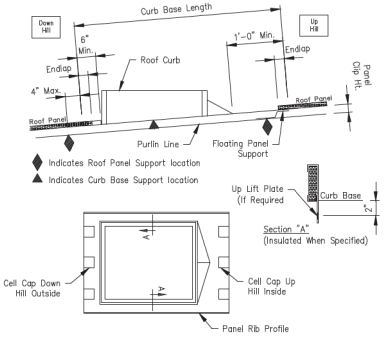
Suggested Method Of Purlin Attachment For Building Accessories



* Denotes Material Not Provided By Metal Building Manufacturer.

The Total Hanger Load Shall Not Exceed The Design Collateral Load For The Building. Example:
5'-0 (Purlin Spacing) X 5'-0 (Hanger Spacing) X 6 PSF (collateral Load) = 150 Lbs.
See Cover Sheet For Design Collateral Load For This Building.
Note: If The Building Is Designed For 0 PSF Collateral Load, Then Adding Any Suspended System (i.e. Duct Work, Piping, Lights, Ceilings, Etc.) Will Correspondingly Reduce The Design Live Load.

Roof Curbs When Not Supplied By Building Manufacturer



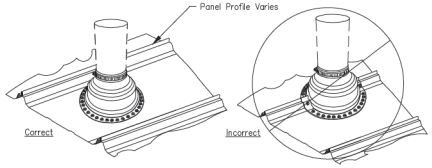
The Curb Details Shown Illustrate The Building Manufacturers Recommended Curb Style And Installation Method. It Is The Erector/Installer's Responsibility To Provide The Proper Curb Style And Install Them In Accordance With The Procedures Established By These Details. Failure By The Erector/Installer To Follow These Recommendations May Result In The Curbs Damaging The Roof System Or Excluded From Warranties.

All Roof Curbs To Be:
1. .080 Aluminum Or 18 Ga. Stainless Steel (No Galvalume® Or Galvanized).
2. Panel Rib To Panel Rib (No Flat Skirt Or Lay-Over Curbs).
3. Installed With Down Hill End Over Panel And Up Hill End Under Panel Application For Water Flow At Panel Splice.
4. Up Lift Prevention For Clip Applied Roof Systems Are Required If:
   a. Wind Loads Exceed 110 MPH.
   b. Curb Base Crosses A Purlin.
5. Supported on (4) Sides By Primary Or Secondary Framing.
6. Maximum Single Curb Weight Recommended Is 1500 Lbs.

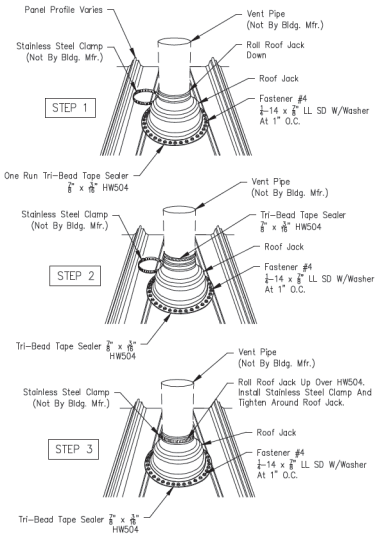
Roof Jack Installation when Not Supplied By Building Manufacturer

General Installation Notes

- Do Not Use Galvanized Roof Jacks, Lead Hats, Or Other Residential Grade Roof Jacks. These Roof Jacks Do Not Have 20 Year Service Life And In Case Of Lead Hats Will Cause Galvanic Corrosion Of The Roof Panel.
- Use EPDM Rubber Roof Jacks With An Integral Aluminum Band Bonded Into The Perimeter Of The Base. EPDM Roof Jacks Have A Temperature Range From −65F To 212F. Use Silicone Roof Jacks For High Temperatures. Silicone Roof Jacks Have A Temperature Range Of −100F To 437F.
- Retrofit Roof Jacks Are Available For Applications In Which The Top Of The Pipe Is Inaccessible, Eliminating The Possibility Of Sliding The Roof Jack Over The Top Of The Pipe.
- Do Not Use Tube Sealant To Seal The Roof Jack To The Roof Panels. Use Roll Tape Sealer Between The Roof Jack And The Roof Panel And Attach The Roof Jack To The Roof Panel With Fastener #4 $\frac{1}{2}$ – 14 X $\frac{3}{8}$" LL SD W/washer At 1" O.C. Around The Base Of The Roof Jack. See Table Below For Quantities.
- Trim The Top Of The Roof Jack To Fit Over The Pipe, Roll Down The Roof Jack Over The Pipe And Apply Tape Sealer For The Perimeter Of The Roof Jack Base Between The Roof Jack And The Roof Panel. Apply Tape Sealer Around The Pipe And Install A Stainless Steel Clamp (Not By Bldg. Mfr.) Over The Top Of The Roof Jack And Firmly Tighten To Form A Secure Compression Seal.
- If The Pipe Diameter Is So Large To Block The Flow Of Water Down The Roof Panel, A Flat Base Roof Curb Must Be Installed Into The Roof And The Roof Jack Will Be Sealed To The Curb. A Two Piece Curb May Be Required When The Top Of The Pipe Is Inaccessible.
- In Northern Climates, The Pipe Penetration Should Be Protected From Moving Ice Or Snow With A Snow Retention System Immediately Up Slope From The Pipe.



Install Pipe In Center To Allow Base Of Roof Jack To Lay Flat on Panel. Cannot Encompass More Than 75% Of Panel.



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